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Louis Bauer, Edward L. Reiss, and Herbert B. Keller

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Louis Bauer, Edward L. Reiss, and Herbert B. Keller<sup>\*</sup>

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<sup>\*</sup>California Institute of Technology.



## ABSTRACT

A nonlinear thin shell theory is derived for the axisymmetric buckling of spherical shells subjected to either a pressure or a centrally directed surface load. The theory is reduced to a boundary value problem for a system of four first order ordinary differential equations. Numerical solutions of this boundary value problem are obtained by the shooting and parallel shooting methods. An extensive numerical study is made of the nonlinear deformations of the shells. We find for example, that all solution branches that bifurcate from the eigenvalues of the linearized buckling theory are connected to each other by means of intermediate branches. Some implications of the numerical results concerning the buckling of spherical shells are discussed.



## 1. Introduction.

The outer surface of a thin-walled hollow sphere\* is subjected to a uniform compressive load  $p$ . It may be either a pressure or a centrally directed load. The uniformly contracted spherical shell is a possible equilibrium state for all values of  $p$ . This state occurs in experiments only for sufficiently small  $p$ . When  $p$  increases to or beyond some critical pressure the shell suddenly jumps or buckles with finite displacements into a non-spherical shape.

The classical or linear buckling theory [1] yields an eigenvalue problem with the eigenvalue parameter proportional to  $p$ . The spectrum is discrete and the lowest eigenvalue is usually called the buckling load. For sufficiently thin shells the eigenfunction or buckling mode consists of many waves over the surface of the shell with an undetermined amplitude. The experimentally measured buckling loads are frequently considerably less than the lowest eigenvalue. The experimentally observed deformation is usually a "dimple" confined to a neighborhood of one point with the remainder of the shell remaining nearly spherical. Kármán and Tsien [2] suggested that a nonlinear theory is required to adequately describe the buckling of spherical shells. They treated the complete spherical shell as a clamped shallow spherical cap corresponding to the region where the dimple occurs. Their analysis was subsequently refined and generalized in [3,4,5,6,7] and elsewhere.\*\* The solutions

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\* For example, a ping-pong ball.

\*\* We refer to the discussion and the extensive bibliographies in [7,8] for references to previous investigations.

of a nonlinear theory have been analyzed in [8,9,15] by a perturbation methods for loads near the eigenvalues of the linearized theory.

In this paper we consider the nonlinear axisymmetric deformations of the hollow sphere. An appropriate shell theory is derived in Section 2 and in Appendix I. The strain displacement relations and the curvature expressions differ from those employed in [8,9]. The shell theory is reduced to a two point boundary value problem, which we call Problem S, for a system of four first order ordinary differential equations. The boundary value problem is the same for pressure or centrally directed loading. We employ the shooting and parallel shooting methods to obtain extensive numerical solutions of Problem S. The numerical methods are described in Section 4. The results of the computations are presented in Section 5. Their implications concerning buckling are discussed in Section 6.

## 2. Formulation.

Let  $r, \theta, \phi$  denote a spherical coordinate system. The radial distance from the origin is  $r$ . The polar angle  $\theta$  is measured from the north pole,  $\theta = 0$ , and is in the range  $0 \leq \theta \leq \pi$ . The circumferential angle  $\phi$  varies in the interval  $0 \leq \phi \leq 2\pi$ . The inner and outer surfaces of the thin spherical shell are given by  $r = R \pm h$  where  $R$  is the radius of the midsurface of the shell and  $2h$  is the uniform thickness. The shell is deformed by a uniform compressive surface load  $p$ .



It is either directed toward the center  $r = 0$  or it is a pressure that is normal to the deformed surface. We show in Appendix I that our shell theory is the same for both loads.

We consider only the axisymmetric deformations of the shell. Thus all displacements and stresses are independent of  $\phi$ . The displacements in the  $\theta$  and the negative  $r$  directions are denoted respectively by  $\bar{U}(r, \theta)$  and  $\bar{W}(r, \theta)$ . The  $\phi$  displacement vanishes because the deformation is axisymmetric. The exact Lagrangian strain displacement relations in spherical coordinates are

$$(2.1a) \quad e_{\theta\theta}^*(r, \theta) = \frac{(\bar{U}_{\theta} - \bar{W})}{r} \left[ 1 + \frac{1}{2} \frac{(\bar{U}_{\theta} - \bar{W})}{r} \right] + \frac{1}{2} \left[ \frac{\bar{U} + \bar{W}}{r} \right]^2 ,$$

$$(2.1b) \quad e_{\phi\phi}^*(r, \theta) = \frac{(\bar{U} \cot \theta - \bar{W})}{r} \left[ 1 + \frac{1}{2} \frac{(\bar{U} \cot \theta - \bar{W})}{r} \right] ,$$

$$(2.1c) \quad e_{r\theta}^*(r, \theta) = \frac{1}{2} \left[ \frac{(\bar{U} + \bar{W})_{\theta}}{r} (\bar{W}_r - 1) + \bar{U}_r \left( 1 + \frac{\bar{U}_{\theta} - \bar{W}}{r} \right) \right] .$$

Here the subscripts on  $e^*$  signify the components of strain. The subscripts on  $\bar{U}$  and  $\bar{W}$  denote partial differentiation. The shear strains  $e_{r\phi}^*$  and  $e_{\theta\phi}^*$  vanish because the deformations are axisymmetric. The strain  $e_r^*$  is not included since it is subsequently neglected in deriving the shell theory.

We assume that the displacements are small but finite. Specifically, this means that

$$(2.2) \quad \frac{|\bar{U}_{\theta} - \bar{W}|}{r} , \quad \frac{|\bar{U} \cot \theta - \bar{W}|}{r} , \quad |\bar{W}_r| \ll 1 .$$

We use (2.2) in (2.1) and obtain the approximate nonlinear strain displacement relations,

$$(2.3a) \quad e_{\theta\theta}^* \approx E_{\theta}(r, \theta) \equiv \frac{\bar{U}_{\theta} - \bar{W}}{r} + \frac{1}{2} \left( \frac{\bar{U} + \bar{W}_{\theta}}{r} \right)^2 ,$$

$$(2.3b) \quad e_{\phi\phi}^* \approx E_{\phi}(r, \theta) \equiv \frac{\bar{U} \cot \theta - \bar{W}}{r} ,$$

$$(2.3c) \quad e_{r\theta}^* \approx E_{r\theta}(r, \theta) \equiv \frac{1}{2} \left[ \bar{U}_r - \frac{(\bar{U} + \bar{W}_{\theta})}{r} \right] .$$

The customary assumptions of shell theory are that: the shell is thin, i.e.  $h/r \ll 1$ ; normals to the midsurface remain normal to the deformed midsurface; the normal stress in the  $r$  direction is negligible compared to the other normal stresses; the absolute values of the strains  $\ll 1$ ; the two dimensional Hooke's law is valid. Thus we define a new dependent variable  $z$  by

$$(2.4) \quad z = R - r , \quad |z| \leq h ,$$

and we assume that

$$(2.5) \quad \bar{W}(r, \theta) = RW(\theta) , \quad \bar{U}(r, \theta) = RU(\theta) - zV(\theta) .$$

Here  $U$  and  $W$  are dimensionless midsurface displacements and  $V$  is proportional to the angle of rotation of a tangent to a meridian. Inserting (2.5) in (2.3c) and setting  $E_{r\theta} = 0$  on  $z = 0$  we obtain

$$(2.6) \quad V = U + W' ,$$

where a prime denotes differentiation with respect to  $\theta$ .

By substituting (2.4)-(2.6) in (2.3a,b) we obtain, since  $|z/R| \ll 1$ ,

$$(2.7) \quad \begin{aligned} E_{\theta}(z, \theta) &= e_{\theta}(\theta) - z\kappa_{\theta}(\theta) , \\ E_{\phi}(z, \theta) &= e_{\phi}(\theta) - z\kappa_{\phi}(\theta) . \end{aligned}$$

Here the midsurface strains,  $e_{\theta}$  and  $e_{\phi}$ , and the curvatures,  $\kappa_{\theta}$  and  $\kappa_{\phi}$ , are defined by

$$(2.8a) \quad e_{\theta} \equiv U' - W + \frac{1}{2} V^2 , \quad e_{\phi} \equiv U \cot \theta - W ,$$

$$(2.8b) \quad \kappa_{\theta} \equiv V'/R , \quad \kappa_{\phi} \equiv (V/R) \cot \theta .$$

The relations (2.8a) have been previously employed by Friedrichs [4] but the curvature expressions (2.8b) differ from those in [4]. Koiter [8], Thompson [9] and others use (2.8) with  $V$  replaced by  $W'$ . The strain displacement equations previously used in analyzing spherical caps, see e.g. [10], are obtained from (2.8) by replacing  $V$  by  $W'$  and  $\cot \theta$  by  $1/\theta$ .

The shell equilibrium equations and boundary conditions are derived in Appendix I using the principle of minimum potential energy. They are reduced to a boundary value problem for the following system of four first order ordinary differential equations:

$$(2.9a) \quad \underline{y}'(\theta) = \underline{f}(\underline{y}, \theta) , \quad 0 \leq \theta \leq \pi .$$

Here  $\underline{y}(\theta)$  is a four dimensional vector with components  $y_i(\theta)$ ,  $i = 1,2,3,4$  and  $\underline{f}(\underline{y},\theta)$  is the four dimensional vector with components  $f_i$ ,  $i = 1,2,3,4$  defined by

$$(2.9b) \quad \begin{aligned} f_1 &\equiv -(1-\nu)y_1 \cot \theta + y_2 + (k \cot^2 \theta - P)y_4 + y_2 y_4 \cot \theta , \\ f_2 &\equiv y_3 , \\ f_3 &\equiv y_2(\cot^2 \theta - \nu) - y_3 \cot \theta - y_4 - y_4^2(\cot \theta)/2 , \\ f_4 &\equiv (1-\nu^2)y_1/k - \nu y_4 \cot \theta . \end{aligned}$$

The parameters  $P$  and  $k$  are defined by

$$(2.9c) \quad P \equiv 2^{-1}(p/E)(R/2h) , \quad k \equiv 3^{-1}(h/R)^2 ,$$

where  $E$  is Young's modulus and  $\nu$  is Poisson's ratio. We refer to  $P$  as the load. The components of  $\underline{y}$  are defined in terms of physical quantities by

$$(2.10) \quad y_1 = m(\theta), \quad y_2 = q(\theta), \quad y_3 = t(\theta), \quad y_4 = v(\theta) \equiv V$$

where  $m$ ,  $q$ ,  $t$  are proportional, respectively, to the radial bending moment, the transverse shear, the circumferential membrane stress; see equations (A.2), (A.15c) and (A.18) in Appendix I.

Since the stresses are bounded and the polar axis is a line of symmetry we have the boundary conditions

$$(2.11) \quad u = w' = q = 0 \quad \text{at} \quad \theta = 0, \pi .$$

Here  $u(\theta) \equiv U(\theta)$  and  $w$  is defined by

$$(2.12) \quad w(\theta) \equiv W(\theta) - W_0 , \quad W_0 \equiv (1-\nu)P .$$

The quantity  $W_0$  is the radial displacement of the uniformly contracted or unbuckled state. From (2.6), (2.10) and (2.12)

$$(2.13) \quad v \equiv u + w' ,$$

so that (2.11) and (2.10) imply that

$$(2.14) \quad y_2 = y_4 = 0 , \quad \theta = 0, \pi .$$

We refer to the boundary value problem (2.9) and (2.14) as Problem S.

If  $y_1$  and  $y_3$  are eliminated from (2.9) we obtain an equivalent system of two second order equations in  $y_2 = q$  and  $y_4 = v$ :

$$(2.15a) \quad Lq + vq = -v - (v^2/2) \cot \theta ,$$

$$(2.15b) \quad Lv - vv = [(1-v^2)/k][-Pv + q + qv \cot \theta] ,$$

where the linear differential operator  $L$  is defined by

$$(2.15c) \quad Lf \equiv f'' + f' \cot \theta - f \cot^2 \theta .$$

The equations of the classical linear theory of bending of spherical shells [11] are obtained from (2.15) by setting  $P = 0$  and omitting the nonlinear terms in (2.15a,b). It is not difficult to show that (2.15) or (2.9) reduce to the differential equations of the nonlinear spherical membrane [12] when we formally set  $k = 0$ . After multiplying (2.15b) by  $k$  and setting  $k = 0$ , the left hand side of (2.15b) formally vanishes if  $\theta$  is bounded away from 0 and  $\pi$  and the order of the system (2.15)

is reduced. The reduction is not uniform in  $\theta$  because of the singularity of  $\cot \theta$  at  $\theta = 0, \pi$ . Thus  $\theta = 0, \pi$  are, in some sense, turning points of (2.15) and boundary layers may form at  $\theta = 0$  or  $\theta = \pi$  (or both) as  $k \rightarrow 0$ . Friedrichs [4], Altshuler [5] and to some extent Gabril'iants and Feodos'ev [6] attempted to analyze this boundary layer.

It is easy to show that if  $|u| \ll |w'|$  and  $\theta \ll 1$  (so that  $v$  is negligible compared to  $\theta^{-2}$ ), (2.15) can be reduced to the spherical cap equations [10].

If  $\underline{y}^0(\theta)$  is a solution of the hemisphere problem,

$$(2.16a) \quad \underline{y}^{0'} = f(\underline{y}^0, \theta) , \quad 0 \leq \theta \leq \pi/2 ,$$

$$(2.16b) \quad \underline{y}_2^0 = \underline{y}_4^0 = 0 \quad , \quad \text{at } \theta = 0 , \pi/2 ,$$

then

$$(2.17) \quad \underline{y}(\theta) = \begin{cases} \underline{y}^0(\theta) & , \quad 0 \leq \theta \leq \pi/2 , \\ \underline{y}^0(\pi-\theta) & , \quad \pi/2 \leq \theta \leq \pi , \end{cases}$$

is a solution of Problem S. Such a solution, which is symmetric with respect to the equatorial plane, is referred to as a symmetric solution. The hemisphere problem (2.16) is also physically significant. The boundary conditions (2.16b) are consequences of the physical conditions  $u(\pi/2) = w'(\pi/2) = q(\pi/2) = 0$ . They imply that the edge of the hemisphere cannot rotate or deflect normal to the equatorial plane but it can slide freely tangential to the plane.

We refer to solutions of Problem S which do not satisfy

$y(\theta) = y(\pi-\theta)$  as unsymmetric solutions.<sup>\*</sup> If  $y^1(\theta)$  is an unsymmetric solution then another distinct solution is  $y^2(\theta) \equiv y^1(\pi-\theta)$ . The solution  $y^2$  is obtained from  $y^1$  by rotating the sphere so that the north and south poles are interchanged. Thus unsymmetric solutions occur in pairs.

When a solution of Problem S has been obtained  $u$  is determined from,<sup>\*\*</sup>

$$(2.18) \quad u' - u \cot \theta = (1+v)(q \cot \theta - t) - v^2/2 ,$$

and  $w$  from (2.13). Equation (2.18) is singular at  $\theta = 0, \pi$ . Therefore the boundary conditions  $u(0) = 0$   $u(\pi) = 0$  are insufficient to uniquely solve (2.18). Unique solutions of (2.18) are obtained by specifying  $u'(0)$  such that  $u(\pi/2) = 0$ .

This is the correct boundary condition for the hemisphere problem. It fixes the rigid body displacement in the complete sphere problem.

The difference between the potential energy of a buckled solution and the unbuckled solution is proportional to

$$(2.19) \quad e = \int_0^\pi [t^2 + q^2(\cot^2\theta - v) + (1-v^2)m^2/k + kv^2\cot^2\theta - 4Pw] \sin \theta \, d\theta.$$

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<sup>\*</sup>They are unsymmetric with respect to the equator, but still retain their axial symmetry.

<sup>\*\*</sup>See Appendix I.

### 3. The Linear Buckling Theory.

The linear buckling theory is obtained by omitting the nonlinear terms  $(v^2/2) \cot \theta$  in (2.15a) and  $qv \cot \theta$  in (2.15b). The resulting eigenvalue problem has non-zero solutions if and only if

$$(3.1a) \quad P = P_n(k) \equiv [k/(1-v^2)][\lambda_n + 1 + v] + 1/[\lambda_n + 1 - v],$$
$$n = 1, 2, \dots,$$

where

$$(3.1b) \quad \lambda_n \equiv n(n+1) - 2.$$

The corresponding eigenfunctions are

$$(3.2) \quad q(\theta) = \alpha_n A_n^{(1)}(\theta), \quad v(\theta) = \beta_n A_n^{(1)}(\theta),$$

$$\alpha_n \equiv \beta_n / [\lambda_n + 1 - v], \quad n = 1, 2, \dots,$$

where  $\beta_n \neq 0$  are arbitrary constants and  $A_n^{(1)}(\theta)$  is the Associated Legendre Function of the first kind of degree  $n$  and order 1, [13]. If  $n$  is even the corresponding eigenfunction is symmetric with respect to  $\theta = \pi/2$  and if  $P_n$  is simple, i.e.  $P_n \neq P_m$  for any  $n \neq m$ , it is called a symmetric eigenvalue. If  $n$  is odd then the eigenfunction is anti-symmetric and if  $P_n$  is simple it is called an unsymmetric eigenvalue. The value of  $n$  for which  $P_n$  is a minimum depends on  $k$ . It may correspond to either a symmetric or an unsymmetric eigenvalue.

The eigenvalues obtained from previous linearized theories [1], differ slightly from (1). For example, the eigenvalues



determined by Zoelly and Schwerin [1] which we denote by  $P_n'$  are given by

$$(3.3) \quad P_n' \equiv \frac{[\lambda_n^2 + 2\lambda_n + (1+v)^2]}{(\lambda_n + 1 + 3v)} \left( \frac{k}{1-v^2} \right) + (\lambda_n + 1 + 3v)^{-1}, \quad n=1,2,\dots$$

For  $n$  large we obtain from (3.1) and (3.3) that

$$P_n = \lambda_n \frac{k}{1-v^2} + \frac{1}{\lambda_n} + (1+v) \frac{k}{1-v^2} + \dots$$

$$P_n' = \lambda_n \frac{k}{1-v^2} + \frac{1}{\lambda_n} + [(1-3v) + \frac{9v^2+v}{\lambda_n}] \frac{k}{1-v^2} + \dots$$

Thus, for large  $n$ ,  $P_n$  is only slightly larger than  $P_n'$  since  $k$  is small.

In the numerical computations described in the following sections we obtain solutions for a wide range of  $P$  values for  $k = 10^{-3}$  and  $k = 10^{-5}$ . In Table I we list, in increasing magnitude, the fifteen lowest eigenvalues computed from (3.1) and (3.3) for these values of  $k$ .

For each of these values of  $k$  the lowest eigenvalue is unsymmetric. There are other values of  $k$ , e.g.  $k = 1.2 \times 10^{-5}$ , for which the lowest eigenvalue is symmetric. There are even values of  $k = k_{mn} \equiv \frac{1-v^2}{(\lambda_n + 1 - v)(\lambda_m + 1 - v)}$  for which the corresponding eigenvalue is a multiple eigenvalue. For example, with  $k = .76935 \times 10^{-3}$ , the lowest eigenvalue occurs for  $n = 5$  and  $n = 6$ .

Table IA,  $k = 10^{-3}$

$n$	$P_n$	$P'_n$
5	.0675324	.0646786
6	.0706160	.0684843
4	.0750573	.0702911
7	.0799192	.0781080
8	.0936046	.0919527
3	.106244	.0949524
9	.110786	.109220
10	.130993	.129476
11	.153954	.152467
12	.179504	.178037
13	.207540	.206085
2	.219602	.172597
14	.237992	.236545
15	.270812	.269371
16	.305967	.304531

Table IB,  $k = 10^{-5}$

$n$	$P_n \times 100$	$P'_n \times 100$
17	.668365	.665572
16	.671713	.668555
18	.673789	.671270
15	.685593	.681940
19	.686669	.684358
20	.706003	.703853
14	.712402	.708064
21	.731019	.728995
13	.755470	.750161
22	.761113	.759190
23	.795809	.793966
12	.819535	.812815
24	.834726	.832946
25	.877555	.875828
11	.911529	.902695

#### 4. Numerical Methods.

For both the sphere and the hemisphere problems, we must solve (2.9) subject to boundary conditions of the form

$$(4.1) \quad y_2(0) = y_4(0) = 0, \quad y_2(L) = y_4(L) = 0.$$

For the hemisphere  $L = \pi/2$  and for the full sphere  $L = \pi$ .

These problems are solved numerically by the shooting method for the hemisphere, and a parallel shooting method for the full sphere. A crucial step is a continuation procedure used to get good estimates of the proper initial data. First we describe our method for the hemisphere problem and then indicate how that procedure is employed to solve the full sphere case. The numerical procedures and the continuation scheme are analyzed rather thoroughly in [14].

##### 4.1 Shooting for Hemispheres.

Consider the initial value problem, with fixed  $P$  and  $k$ ,

$$(4.2a) \quad \ddot{u}'(\theta) = \ddot{f}(u, \theta),$$

$$(4.2b) \quad u(0) = \begin{pmatrix} \xi_1 \\ 0 \\ \xi_2 \\ 0 \end{pmatrix}.$$

We denote the solution of (4.2) by  $\ddot{u} = \ddot{u}(\theta; \xi_1, \xi_2)$ . If this solution exists in  $0 \leq \theta \leq \pi/2$  for initial parameters  $\xi_1$  and  $\xi_2$  such that

$$(4.3) \quad u_2(\pi/2; \xi_1, \xi_2) = 0, \quad u_4(\pi/2; \xi_1, \xi_2) = 0,$$

then by (4.1),  $\underline{y}(\theta) \equiv \underline{u}(\theta; \xi_1, \xi_2)$  is a solution of the hemisphere problem. The number of distinct solutions of the hemisphere problem is equal to the number of distinct real roots  $(\xi_1, \xi_2)$  of (4.3). These roots are numerically determined using Newton's method, and a continuity procedure. For this purpose we introduce the vectors

$$(4.4a) \quad \underline{\tilde{u}}^{(1)}(\theta) \equiv \frac{\partial \underline{u}}{\partial \xi_1}, \quad \underline{\tilde{u}}^{(2)}(\theta) \equiv \frac{\partial \underline{u}}{\partial \xi_2}.$$

Then it follows by differentiation in (4.2) that the  $\underline{\tilde{u}}^{(j)}$  satisfy the linear variational systems:

$$(4.4b) \quad \frac{d\underline{\tilde{u}}^{(1)}}{d\theta} = A(\underline{u}, \theta) \underline{\tilde{u}}^{(1)}, \quad \underline{\tilde{u}}^{(1)}(0) = \underline{\tilde{e}}^{(1)} = \begin{pmatrix} 1 \\ 0 \\ 0 \\ 0 \end{pmatrix};$$

$$(4.4c) \quad \frac{d\underline{\tilde{u}}^{(2)}}{d\theta} = A(\underline{u}, \theta) \underline{\tilde{u}}^{(2)}, \quad \underline{\tilde{u}}^{(2)}(0) = \underline{\tilde{e}}^{(2)} = \begin{pmatrix} 0 \\ 0 \\ 1 \\ 0 \end{pmatrix}.$$

Here  $A$  is the fourth order Jacobian matrix,

$$(4.4d) \quad A(\underline{u}, \theta) \equiv \left( \frac{\partial f_i(\underline{u}, \theta)}{\partial u_j} \right).$$

Given an initial estimate  $(\xi_1^{(0)}, \xi_2^{(0)})$  for a root of (4.3), the sequence of Newton iterates  $\{\xi_1^{(v)}, \xi_2^{(v)}\}$  is defined by

$$(4.5a) \quad \xi_j^{(v+1)} = \xi_j^{(v)} + \Delta \xi_j^{(v)}; \quad j=1,2; \quad v=0,1,2\dots$$

where  $(\Delta \xi_1^{(v)}, \Delta \xi_2^{(v)})$  is the solution of the linear system

$$(4.5b) \quad \begin{aligned} U_2^{(1)}(\pi/2) \Delta \xi_1^{(v)} + U_2^{(2)}(\pi/2) \Delta \xi_2^{(v)} &= -u_2(\pi/2; \xi_1^{(v)}, \xi_2^{(v)}) , \\ U_4^{(1)}(\pi/2) \Delta \xi_1^{(v)} + U_4^{(2)}(\pi/2) \Delta \xi_2^{(v)} &= -u_4(\pi/2; \xi_1^{(v)}, \xi_2^{(v)}) . \end{aligned}$$

The coefficient elements  $U_j^{(i)}(\pi/2)$  in (4.5b) are the corresponding elements in the solution of (4.4) in which  $\underline{u}(\theta; \xi_1^{(v)}, \xi_2^{(v)})$  is used in the evaluation of  $A(\underline{u}, \theta)$ .

A Runge-Kutta method is employed to solve the three initial value problems (4.2), (4.4b) and (4.4c). The singularity at  $\theta = 0$  requires special starting procedures. In (4.2a) we simply use the limiting relations

$$(4.6) \quad \underline{f}(\underline{u}(0), 0) = \begin{pmatrix} 0 \\ \xi_2 \\ 0 \\ \frac{1-v}{k} \xi_1 \end{pmatrix}$$

obtained from (2.9b) and (4.2) by requiring continuous derivatives at  $\theta = 0$ . In (4.4b) and (4.4c) the first step in the integration is done with the centered scheme:

$$(4.7) \quad \underline{U}(h) = \underline{U}(0) + \frac{h}{2} A\left(\frac{1}{2}[\underline{u}(0) + \underline{u}(h)], \frac{h}{2}\right) \cdot [\underline{U}(0) + \underline{U}(h)]$$

where  $h$  is the step width. The error is  $O(h^3)$  since we use this second order scheme only at one point.

The interval  $0 < \theta < \pi/2$  is divided into 100 equal subintervals and the integrations are performed, starting at

$\theta = 0$ , over each subinterval with spacings  $h_0 = \pi/200$ , and  $h_1 = h_0/2$ . If the maximum relative difference in the two integrations at the end of a subinterval is not less than  $10^{-M}$ , we then halve the step width, and repeat the procedure over the subinterval. When agreement is achieved we proceed to the next subinterval, starting with the original mesh widths  $h_0$  and  $h_1$ . The iterates in Newton's method (4.5) are terminated when  $\max_{i=1,2} |\Delta \xi_i^{(v)} / \xi_i^{(v)}| < 10^{-M}$ . In most of the calculations  $M = 7$ . Occasionally a larger value of  $M$  is used.

#### 4.2 Continuation Method for Parameter Variation.

After a solution is determined for some value of  $P$  with  $k$  fixed, (or for some value of  $k$  with  $P$  fixed) accurate estimates of the initial iterates  $\xi_j^{(0)}$  to use for  $P \pm \Delta P$  (or for  $k \pm \Delta k$ ) can be obtained by finding  $\partial \xi_j / \partial P$  (or  $\partial \xi_j / \partial k$ ). With  $k$  fixed let  $\xi_j(P)$  denote the exact initial values for any given  $P$ . Then the boundary conditions (4.3) yield the identities in  $P$ :

$$(4.3') \quad \begin{aligned} u_2(\pi/2; \xi_1(P), \xi_2(P), P) &= 0, \\ u_4(\pi/2; \xi_1(P), \xi_2(P), P) &= 0. \end{aligned}$$

In rewriting (4.3) we have indicated the dependence of  $\underline{u}$  on  $P$  which was previously ignored to simplify the notation. Differentiating each of these identities with respect to  $P$ , we obtain, recalling (4.4a),

$$(4.8) \quad U_2^{(1)}(\pi/2)\dot{\xi}_1(P) + U_2^{(2)}(\pi/2)\dot{\xi}_2(P) = -U_2^{(5)}(\pi/2) ,$$

$$U_4^{(1)}(\pi/2)\dot{\xi}_1(P) + U_4^{(2)}(\pi/2)\dot{\xi}_2(P) = -U_4^{(5)}(\pi/2) ,$$

where we have introduced

$$(4.9a) \quad \underline{\tilde{U}}^{(5)}(\theta) \equiv \frac{\partial \underline{\tilde{u}}}{\partial \underline{\tilde{P}}} , \quad \dot{\xi}_i(P) \equiv \frac{\partial \xi_i(P)}{\partial P} , \quad i = 1, 2.$$

By differentiating in (4.2) we find with the aid of (2.9b) and (4.4d) that

$$(4.9b) \quad \frac{d\underline{\tilde{U}}^{(5)}(\theta)}{d\theta} = A(\underline{\tilde{u}}, \theta)\underline{\tilde{U}}^{(5)} + \begin{bmatrix} -u_4(\theta) \\ 0 \\ 0 \\ 0 \end{bmatrix} , \quad \underline{\tilde{U}}^{(5)}(0) = \underline{0} .$$

The coefficients in (4.8) are already computed for use in the final Newton iterate (4.5b). Thus only the additional system (4.9b) is solved to evaluate  $\dot{\xi}_1$  and  $\dot{\xi}_2$  from (4.8). Then for initial estimates we use the first two terms in the Taylor expansion:

$$(4.10) \quad \xi_j^{(0)}(P \pm \Delta P) = \xi_j(P) \pm \dot{\xi}_j(P)\Delta P , \quad j = 1, 2.$$

The derivatives  $\dot{\xi}_j(P)$  are of physical interest, especially when  $P$  is near a bifurcation point or near a local maximum or minimum point of  $P(\xi_j)$ .

The corresponding continuation analysis and computation for variation of the geometric parameter  $k$  follow in an obvious manner. In fact, to replace  $P$  by  $k$  only the right hand side in (4.8) need be changed by using the components of

$$(4.11a) \quad \tilde{u}^{(6)}(\theta) \equiv \frac{\partial \tilde{u}}{\partial k} \quad .$$

Proceeding the same way as in the derivation of (4.9b) we find that

$$(4.11b) \quad \frac{d\tilde{u}^{(6)}(\theta)}{d\theta} = A(\tilde{u}, \theta) \tilde{u}^{(6)}(\theta) + \begin{pmatrix} u_4(\theta) \cot^2 \theta \\ 0 \\ 0 \\ -\frac{1-v^2}{k^2} u_1(\theta) \end{pmatrix}, \quad \tilde{u}^{(6)}(0) = 0.$$

### 4.3 Parallel Shooting for the Full Sphere.

We treat the full sphere by employing the previous procedure for two hemispheres.\* We integrate from each pole to the equator and adjust four parameters in order to satisfy four continuity conditions at the equator.

Consider the two initial value problems,

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\* It is inconvenient to treat the full sphere by integrating from  $\theta = 0$  and adjusting  $\xi_1$  and  $\xi_2$  to satisfy the two boundary conditions at  $\theta = \pi$ . Some of the derivatives are indeterminate at  $\theta = \pi$ , and the derivatives of some of the auxiliary variables  $\tilde{u}^{(1)}$  and  $\tilde{u}^{(2)}$  are infinite there. Numerically, these singularities are handled more readily by the special starting procedure, (4.6) and (4.7), than by integrating up to  $\pi - \epsilon$ , and then using special formulas.



$$(4.12a) \quad \frac{d\tilde{u}}{d\theta} = \tilde{f}(\tilde{u}, \theta) , \quad \tilde{u}(0) = \begin{pmatrix} \xi_1 \\ 0 \\ \xi_2 \\ 0 \end{pmatrix} , \quad 0 \leq \theta \leq \pi/2;$$

$$(4.12b) \quad \frac{d\tilde{v}}{d\theta} = \tilde{f}(\tilde{v}, \theta) , \quad \tilde{v}(\pi) = \begin{pmatrix} \xi_3 \\ 0 \\ \xi_4 \\ 0 \end{pmatrix} , \quad \pi \geq \theta \geq \pi/2.$$

Here the  $\xi_i$  are the four initial parameters, two for each of the two initial value problems. The solutions of these problems are denoted by  $\tilde{u}(\theta; \xi_1, \xi_2)$  and  $\tilde{v}(\theta; \xi_3, \xi_4)$ . If we can find  $\xi_i$  such that

$$(4.13) \quad u_j(\pi/2; \xi_1, \xi_2) = v_j(\pi/2; \xi_3, \xi_4) , \quad j = 1, 2, 3, 4,$$

then a solution of the full sphere problem is given by

$$(4.14) \quad \tilde{y}(\theta) \equiv \begin{cases} \tilde{u}(\theta; \xi_1, \xi_2) , & 0 \leq \theta \leq \pi/2, \\ \tilde{v}(\theta; \xi_3, \xi_4) , & \pi/2 \leq \theta \leq \pi . \end{cases}$$

For fixed  $P$  and  $k$ , the number of distinct solutions of Problem S is equal to the number of distinct real roots  $(\xi_1, \xi_2, \xi_3, \xi_4)$  of (4.13).

The equations (4.13) are also solved by Newton's method. Now however there are two nonlinear systems (4.12) and four linear variational systems to be solved for each Newton iteration step. The numerical integration of the initial value

problems is done in the same way as for the hemisphere. All convergence criteria are the same as in the hemisphere calculations. Corresponding continuity methods in both  $k$  and  $P$  are also used for obtaining initial iterates.

#### 4.4 Parallel Shooting for Hemispheres.

For some of the solution branches for the hemisphere problem boundary layers occur at  $\theta = 0$  or  $\theta = \pi/2$ ; see the discussion in Section 5. When a boundary layer occurs at  $\theta = \pi/2$  the solution varies rapidly in a small neighborhood of  $\theta = \pi/2$ . Thus small variations in the parameters  $\xi_1$  and  $\xi_2$  produce large variations in the solution of (4.2) at  $\theta = \pi/2$ . When a boundary layer occurs at  $\theta = 0$ ,  $u_2$  and  $u_4$  essentially vanish in an interval  $a < \theta \leq \pi/2$ . Then relatively large changes in  $\xi_1$  and  $\xi_2$  produce small changes in the solution at  $\theta = \pi/2$ .

In both cases the simple shooting method of Section 4.1 is unsatisfactory for an accurate determination of the roots of (4.3) and parallel shooting is employed. Thus we select a point  $\theta_0$  in the interval  $0 < \theta_0 < \pi/2$  and integrate from  $\theta = 0$  to  $\theta_0$  and from  $\theta = \pi/2$  to  $\theta_0$ . The corresponding four initial parameters are determined from continuity requirements analogous to (4.13). The procedure is completely analogous to the parallel shooting for the full sphere described in Section 4.3. Appropriate values of  $\theta_0$  are determined from numerical experiments.

## 5. Presentation of Results.

Extensive numerical solutions of Problem S have been obtained for shells with  $k = 10^{-3}$  (the thick sphere) and  $k=10^{-5}$  (the thin sphere) over a wide range of  $P$  values. For  $k=1.2 \times 10^{-5}$  a less extensive  $P$  range has been covered. At the fixed pressure  $P = 6 \times 10^{-4}$  solutions have been obtained for various  $k$  in the range  $10^{-5} \leq k \leq 2 \times 10^{-6}$ . We shall describe the thick sphere results that seem most relevant to buckling and briefly mention significant differences for the thin sphere case. The implications of these results for axisymmetric buckling are discussed in Section 6.

The thick sphere computations are summarized in Figures 1 and 2 where we plot  $P$  vs.  $A(P)$  and  $e(P)$ . The quantity

$$(5.1) \quad A(P) \equiv \left( \frac{1}{2} \int_0^\pi w^2 d\theta \right)^{1/2} \geq 0$$

is a measure of the amplitude of the deviation of the deformation at load  $P$  from the uniformly compressed state,  $w \equiv 0$ . Similarly  $e(P)$ , defined in (2.19), is proportional to the difference between the potential energies of the buckled and unbuckled states at the same load,  $P$ . The circles and crosses on the  $P$ -axis denote, respectively, the symmetric and unsymmetric eigenvalues of the linear theory (see Table IA in Section 3).

Each point  $(A(P), P)$  with  $A(P) > 0$  on the graphs in Figure 1 represents buckled states, i.e. nontrivial solutions of Problem S. If the point corresponds to a symmetric state,

then it represents a single solution. If it corresponds to an unsymmetric state, then it represents a pair of solutions. The graphs show that for fixed values of  $P$ , Problem S may have many solutions. To describe and discuss this multiplicity, we decompose the graphs of  $A(P)$  into single valued segments of maximal extent and denote each such segment by  $B_{n,j}$  with integers  $n$  and  $j$ . The procedure for choosing appropriate values of  $n$  and  $j$  is described in Section 5.1. This is equivalent to decomposing the graph of  $e(P)$  in Figure 2 into monotone segments of maximal extent. The  $P$  coordinates of the end points of the segments are denoted by  $P_{n,j}^L$  and  $P_{n,j}^U$ . We call them, respectively, the lower and upper critical points of the segment. If each point on  $B_{n,j}$  corresponds to a symmetric [unsymmetric] solution, then we denote the branch of solutions corresponding to  $B_{n,j}$  as  $B_{n,j}(P)$  [ $B_{n,j}^{\pm}(P)$ ]. The  $P$  axis corresponds to the unbuckled states. We shall not include this branch in our discussion of the multiplicity.

Buckled solutions of Problem S are said to bifurcate from the unbuckled solution at or from a pressure  $P_0$  if there is a solution branch,  $\underline{y}(\theta, P)$  depending continuously on  $P$  such that  $\underline{y}(\theta, P_0) \equiv 0$  and  $\underline{y}(\theta, P) \not\equiv 0$  for  $P \neq P_0$  but  $|P - P_0|$  sufficiently small. This solution branch is called a bifurcation branch. The bifurcation branch need not exist for  $P$  in a full neighborhood of  $P_0$ . If it exists, locally, for  $P > P_0$  ( $< P_0$ ) we say that solutions bifurcate or branch up (down) from  $P_0$ .

Our calculations show that there are solutions in a

full neighborhood of each of the symmetric eigenvalues that were analyzed and that solutions branch either up or down from each of the unsymmetric eigenvalues that were analyzed. Furthermore the results indicate that  $\phi \equiv (dA(P_n)/dP)^{-1} = 0$  for  $n$  odd, i.e. unsymmetric eigenvalues, but not for  $n$  even and  $\phi$  is independent of  $k$ . These results are in agreement with the conclusions of bifurcation theory [15].

### 5.1 The Thick Sphere, $k = 10^{-3}$ .

Perhaps the most striking observation in the thick sphere case is the fact that all computed branches of solutions, 21 symmetric branches and 16 pairs of unsymmetric branches, are connected. Furthermore, for each branch  $B_{n,j}$  the corresponding potential energy  $e_{n,j}(P)$  is a monotone decreasing function of  $P$  on  $P_{n,j}^L \leq P \leq P_{n,j}^U$  (see Figure 2a). All but two of the symmetric branches and five pairs of the unsymmetric branches contained pressures  $P = P_{n,j}^E$ , called equal energy loads, at which  $e_{n,j}(P_{n,j}^E) = 0$  for a buckled state. The critical pressures, equal energy loads and connectivity of the unsymmetric branches and the first 16 symmetric branches of solutions are summarized in Table II. Each branch bifurcating from an eigenvalue is indicated by an asterisk. This table complements Figures 1 and 2.

The branch  $B_{6,1}$  bifurcating up and down from the lowest symmetric eigenvalue  $P_6 \doteq 0.7062$  extends from the lower critical load  $P_{6,1}^L \doteq .01045$  to the upper critical load  $P_{6,1}^U \doteq .07127$ . At its lower endpoint  $B_{6,1}$  joins the branch we call  $B_{6,0}$  whose

Table IIA

## Symmetric Branches

Branch $B_{n,j}$	$P_{n,j}^L$	$P_{n,j}^E$ or $(P_n)$	$P_{n,j}^U$
$B_{6,0}$		.01214	?
	.0104496		
* $B_{6,1}$		(.0706150)	
			.0712660
$B_{6,2}$		.07123	
	.0678306		
* $B_{4,1}$		(.0750573)	
			.0855778
$B_{4,2}$		.0848	
	.0452205		
$B_{4,3}$		---	
			.0662950
$B_{4,4}$		---	
	.04522		
* $B_{8,1}$		(.0936046)	
			.109261
$B_{8,2}$		.1077	
	.0656532		
$B_{8,3}$		.1358	
			.1366638
* $B_{10,1}$		(.130993)	
	.0735912		
* $B_{12,1}$		(.179504)	
			.179807
$B_{12,2}$		.1798	
	.105326		
* $B_{2,1}$		(.219602)	
			.306228
$B_{2,2}$		.23797	
	.237956		
* $B_{14,1}$		(.237992)	
			.4520

Table IIB  
Unsymmetric Branches

Branch $B_{n,j}^+$	$P_{n,j}^L$	$P_{n,j}^E$ or $(P_n)$	$P_{n,j}^U$
* $B_{5,1}$		(.0675324)	.0675324
$B_{5,2}$	.0128125	.0137	.0202242
$B_{5,3}$		---	
$B_{5,4}$	.0173426	---	.0199705
$B_{5,5}$		---	
$B_{5,6}$	.0199473		.0208548
$B_{5,7}$	.0123971 (on $B_{6,1}$ )	.01415	
* $B_{7,1}$	.0799192	(.0799192)	.0858161
$B_{7,2}$		.0855	
$B_{7,3}$	.0467240	---	.064663 (on $B_{4,4}$ )
* $B_{3,1}$	.106244	(.106244)	.110025
$B_{3,2}$		.109531	
$B_{3,3}$	.105617	---	.105790 (on $B_{8,2}$ )
* $B_{9,1}$	.110786	(.110786)	.120483
$B_{9,2}$		.118910	
$B_{9,3}$	.0706469	---	.10349 (on $B_{8,3}$ )



lower endpoint is at  $P_{6,0}^L = P_{6,1}^L$  and whose upper endpoint has not been determined, but is greater than  $P = .1$ ; see Figure 1b. At its upper endpoint  $B_{6,1}$  joins  $B_{6,2}$  whose upper endpoint is at  $P_{6,2}^U = P_{6,1}^U$  and whose lower endpoint is at  $P_{6,2}^L \doteq .06783$ ; see Figures 1a and 2a. The labeling of adjacent joining branches follows the arbitrary rule implied above with the proviso that whenever a bifurcation branch occurs, say bifurcating from  $P = P_n$ , we call it  $B_{n,1}$ . Thus for example, the branch joining  $B_{6,2}$  at its lower critical pressure should be called  $B_{6,3}$ . But since  $B_{6,3}$  bifurcates from  $P_4$  we also call it  $B_{4,1}$ . The notation in Table II is now rather obvious if we merely observe that most critical pressures,  $P_{n,j}^L$  or  $P_{n,j}^U$ , have dual representations. On the branches bifurcating from unsymmetric eigenvalues the eigenvalues are also upper or lower critical pressures, that is,  $P_n = P_{n,1}^L$  or  $P_n = P_{n,1}^U$ .

In Figures 3 and 4 we show how the solutions for  $P$  decreasing on the lowest symmetric bifurcation branch  $B_{6,1}$  go over continuously to the solutions for  $P$  increasing on the adjacent branch  $B_{6,0}$ . There is an obvious difference in the shapes and stresses between the states on branches  $B_{6,1}$  and  $B_{6,0}$  for  $P > 0.05$ ; compare Figures 3a and 4a with Figures 3c and 4c. However, for  $P$  near  $P_{6,1}^L = P_{6,0}^L \doteq 0.0105$  the shapes on both branches are quite similar. Thus there is no abrupt change in mode (see Figures 3b and 4b). As  $P$  increases the states on  $B_{6,0}$  develop a boundary layer near  $\theta = \pi/2$ . For example, at  $P = 0.10$ ,  $t(\theta)$  is almost constant over  $0 \leq \theta \leq \pi/3$  and then



changes rapidly over  $\pi/3 < \theta < \pi/2$  to satisfy the boundary conditions. The graph of  $w(\theta)$  is approximately a parabola with maximum at  $\theta = 0$  so that the deformed sphere is essentially two pushed in hemispheres joined at the equator. The development of the boundary layer and the simple pushed-in shape of the sphere on  $B_{6,0}$  suggests that this mode and branch of solutions continue to exist as  $P \rightarrow +\infty$ . The rapid decay of energy  $e_{6,0}(P)$  would seem to be consistent with this assumption.

In Figures 5 and 6 we show four solutions on the branches  $B_{6,1}$ ,  $B_{6,2}$  and  $B_{6,3} \equiv B_{4,1}$ . One of these solutions is for  $P = .0712594$ , near the critical pressure  $P_{6,1}^U = P_{6,2}^U$ , and another is for  $P = .0678672$  near the critical pressure  $P_{6,2}^L = P_{4,1}^L$ . The other solutions are for  $P$  near the bifurcation loads  $P_6$  and  $P_4$ . These solutions have distinct mode shapes and relatively small amplitudes since  $P$  is close to  $P_4$  or  $P_6$ . This behavior is typical for all even  $n$  since the solutions on different symmetric bifurcation branches  $B_{n,1}$  have distinctly different deformation and stress modes for  $P$  near  $P_n$ , and as  $P$  varies the solutions continuously change on each branch and merge with the solutions on some other branch which is usually not a bifurcation branch.\* Thus all symmetric solutions are connected as  $P$  varies up and down on the appropriate symmetric branches. The energy variation on all branches is, as previously noted, monotone decreasing. However, for  $P < P_2$  the energy on

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\*The only symmetric bifurcation branches to merge directly with each other were  $B_{10,1}$  and  $B_{12,1}$ .

most symmetric branches is mainly positive but for  $P > P_2$  the opposite occurs (the notable exception is  $B_{6,0}$ , see Figure 2a and Table II).

The pairs of unsymmetric bifurcation branches  $B_{n,1}^{\pm}$  merge continuously at the eigenvalues  $P = P_n$  for  $n$  odd. The branches  $B_{5,1}^{\pm}$  bifurcate down from the lowest eigenvalue  $P_5 \doteq 0.0675$  and merge, respectively, with  $B_{5,2}^{\pm}$  at  $P_{5,2}^L \doteq 0.0128$ . Similarly,  $B_{5,2}^{\pm}$  joins  $B_{5,3}^{\pm}$  which joins  $B_{5,4}^{\pm}$  which joins  $B_{5,5}^{\pm}$  which joins  $B_{5,6}^{\pm}$  which joins  $B_{5,7}^{\pm}$  which finally joins  $B_{5,7}^{\pm}$  at  $P_{5,7}^L \doteq 0.0124$ ; see Figure 2b and Table IIB. The obvious mergers also occur for the  $B_{5,j}^{\pm}$ . Since  $B_{5,7}^+$  and  $B_{5,7}^-$  join at  $P_{5,7}^L$  i.e.,  $y^+(\theta, P_{5,7}^L) = y^-(\pi - \theta, P_{5,7}^L) = y^+(\pi - \theta, P_{5,7}^L)$ ,  $B_{5,7}^{\pm}(P)$  must be symmetric at  $P = P_{5,7}^L$ . Furthermore, the results show that this symmetric state lies on the branch  $B_{6,1}$ . The value  $P_{5,7}^L$  is not a critical pressure for  $B_{6,1}$ . In fact, we find that  $P_{6,1}^L < P_{5,7}^L < P_{6,1}^U$ ; see Table II. Equal energy loads are found on the branches  $B_{5,2}^{\pm}$  and  $B_{5,7}^{\pm}$ . Graphs of typical solutions on branch  $B_{5,1}^{\pm}$  are shown in Figures 7a and 8a. Solutions at some of the upper and lower critical loads  $P_{5,j}^U$  and  $P_{5,j}^L$  are shown in Figures 7b and 8b. The transition from one branch to another is indicated by these graphs. We note that on  $B_{5,1}^{\pm}$  the "southern" hemisphere is less distorted than the "northern" hemisphere which suffers a relatively large indentation, or dimple, around the pole. Similar features occur in all the  $B_{5,j}^{\pm}$  branches.

At each of the unsymmetric eigenvalues  $P_n$ , for  $n = 7, 3$  and  $9$  the solutions  $B_{n,1}^{\pm}$  bifurcate up and go through a series

of mergers with intermediate branches. Finally,  $B_{n,j}^+$  joins  $B_{n,j}^-$  for some  $j$  at  $P = P_{n,j}^U$  or  $P = P_{n,j}^L$ . Again the symmetric state into which the unsymmetric pairs of solutions finally merge is found to lie on some symmetric branch of solutions. For example,  $B_{7,3}^+$  join  $B_{4,4}$  at  $P_{7,3}^U \doteq 0.0646$ . Thus we find the amazing result that all bifurcation branches, for both symmetric and unsymmetric modes, are connected to each other by means of intermediate branches. The energies on the bifurcation branches  $B_{n,1}^+$  for  $n = 7,3,9$  are negative for  $P > P_n$ . They are also negative on  $B_{n,2}$  for a limited range of  $P$  values.

## 5.2 The Thin Spheres.

The results for the thin sphere,  $k = 10^{-5}$ , are summarized in Figures 9 and 10. For this value of  $k$  the lowest eigenvalue is also unsymmetric. The eigenvalues for  $k = 10^{-5}$  (see Table IB of Section 3), are closely spaced. For example a 10% change in  $P$  from  $P = P_{17}$  spans seven additional eigenvalues. A 1% change includes two additional eigenvalues. The solutions for the thin sphere are qualitatively similar, in many respects, to those previously discussed for the thick sphere.\* All the branches bifurcating from symmetric eigenvalues are continuously connected in the same way as for the thick sphere. The solution branch

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\* Perhaps the most significant difference is the increase in the multiplicity of the solutions as  $k$  decreases. For example, for the thick sphere 7 different symmetric solutions were determined at  $P = .07$ . For the thin sphere 46 different symmetric solutions were determined at  $P = .007$ .

bifurcates down from the lowest eigenvalue  $P_{17}$ . The solution branches bifurcate up from the next two unsymmetric eigenvalues  $P_{15}$  and  $P_{19}$  and have negative energy for  $P$  near the eigenvalues. We have not completely investigated the unsymmetric branches of the solution for the thin spheres. However, numerical results that are not shown in the graphs indicate that the unsymmetric solutions connect with the symmetric ones in the same way as for the thick sphere. The formation of boundary layers as  $P$  varies on  $B_{16,j}$  is more pronounced for the thin sphere than for the corresponding branch of the thick sphere.

A feature of the thin sphere that has not been observed for the thick sphere is the existence of isolated solution branches. They are branches that are not connected to any of the solutions which bifurcate from the eigenvalues. For example, one of these occurs for  $P$  in the interval  $.006005 < P < .010338$ . There are critical loads at  $P = .007760, .006095, .010338$  and  $.006005$ . The energies on this branch are in the interval  $+.119 < (e/2)^{1/3} < .232$ . Since the energy is large and positive this branch is not shown in Figure 9. We have not found any isolated branches with negative energy.

We have briefly investigated the solutions for the sphere with  $k = 1.2 \times 10^{-5}$  in the neighborhood of the first few eigenvalues. The results are sketched in Figure 11. The lowest eigenvalue is symmetric. The solution branch which bifurcates from this eigenvalue has the same qualitative behavior as the

bifurcation branch from the lowest symmetric eigenvalue for  $k = 10^{-3}$  and  $k = 10^{-5}$ . The second eigenvalue is unsymmetric and the solutions bifurcate up and have negative energy near the eigenvalue. The solutions branch down from the third eigenvalue  $P = P_{15}$ . They are qualitatively similar to the bifurcation branch from the lowest eigenvalue for  $k = 10^{-3}$  and  $k = 10^{-5}$ .

### 5.3 Boundary Layers.

We have observed in Section 5.1, see Figures 3 and 4, that for the bifurcation branch from the lowest symmetric eigenvalue, a boundary layer forms at  $\theta = \pi/2$ . A second boundary layer was observed to develop at  $\theta = 0$  as  $k \rightarrow 0$ ; the solution is small everywhere except at  $\theta = 0$ . In Figures 12 and 13 we show the formation of this boundary layer by presenting graphs of solutions for a sequence of decreasing values of  $k$ ; cf. with Figures 3 and 4. These results partially substantiate Friedrichs' conjectures [4] about the formation of "dimples" as  $k \rightarrow 0$ .

### 6. Remarks about Buckling.

We shall discuss the relevance of the results previously described to the axisymmetric buckling of perfect spherical shells under idealized experimental conditions; that is "gedanken buckling." We discuss primarily the thick sphere since our results are most comprehensive in this case. But our remarks are

equally applicable to the thin sphere and no doubt to a wide range of  $k$  values. Our basic premise is that we have determined all of the relevant axially symmetric equilibrium states of the shell for all loads in some large range:  $0 \leq P \leq P_{\max}$ . Constant reference to Figures 1a and 2 and Table II is required for this discussion.

We imagine the pressure increasing continuously from  $P = 0$  and allow only axisymmetric deformations. The sphere contracts radially (corresponding to the trivial solution  $w \equiv 0$ ) for small  $P$  since this is the only existing equilibrium state. However, as soon as  $P$  exceeds  $P_{6,1}^L = P_{6,0}^L \doteq .01045$ , two additional equilibrium states exist corresponding to the branches  $B_{6,1}$  and  $B_{6,0}$ . Suppose for the present that the shell does not jump or is somehow constrained from jumping to another equilibrium state. Then as the load continues to increase the sphere uniformly contracts, regardless of the numerous states which become possible, until  $P$  finally reaches the lowest eigenvalue. In the thick sphere case this is  $P_5$ . At this pressure it may be possible for the sphere to deviate from perfect sphericity without jumping by merely deforming continuously into the bifurcated mode of solution. In general if the bifurcation branch branches up (down) this smooth transition can occur only for  $P$  increasing (decreasing). Thus since  $B_{5,1}$  branches down from  $P_5$  such a smooth transition cannot occur at  $P_5$  as  $P$  increases.\*

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\* For other values of  $k$ , e.g.  $k = 1.2 \times 10^{-5}$  the lowest eigenvalue is symmetric; see Figure 13. Therefore, a smooth transition into the bifurcation branch as  $P$  increases is possible.



We recall that for the bifurcation branches  $B_{2m,1}$  passing through each symmetric eigenvalue  $P_{2m}$ , the energy  $e_{2m,1}(P)$  is monotone decreasing. Thus as  $P$  increases beyond some  $P_{2m}$  the unbuckled state has more energy than the state on  $B_{2m,1}$ . If we make the reasonable assumption that at a bifurcation point states with least energy are preferred then the thick sphere smoothly deforms into the state on  $B_{6,1}$  as  $P$  goes from  $P_5$  past  $P_6$ . However, suppose the shell remains spherical as  $P$  increases beyond  $P_6$  and  $P_4$ . Then at the unsymmetric eigenvalue  $P_7$ , whose branches  $B_{7,1}^{\pm}$  bifurcate up, the smooth transition can occur onto one of the unsymmetric branches. Thus in summary, if jump-buckling is somehow prevented or just does not occur, the uniformly contracted sphere smoothly deforms into a nonspherical shape as  $P$  increases past a symmetric eigenvalue or an unsymmetric eigenvalue which branches up. We call smooth transitions to nonspherical states, as  $P$  increases, "bifurcation buckling." Then all the symmetric eigenvalues,  $P_{2m}$ , are bifurcation buckling loads and so are those unsymmetric eigenvalues,  $P_{2m+1}$ , with upward branching solutions.

Let us now consider that bifurcation buckling has occurred at  $P = P_n$  and  $P$  continues to increase. The states of deformation correspond to solutions on some upward bifurcating branch  $B_{n,1}$ . However, as  $P$  increases beyond the upper critical point  $P_{n,1}^U$  the deformation cannot vary continuously with  $P$  since the branch  $B_{n,1}$  ceases to exist. Thus the shell must jump to some other non-adjacent equilibrium state. We therefore

call  $P_{n,1}^U$  a jump-buckling load. Clearly,  $P_{2m,1}^U$  are all jump-buckling loads and so are those  $P_{2m+1,1}^U$  which are endpoints of upward bifurcating branches. For example,  $P_{7,1}^U \doteq 0.0858$  for the thick sphere is a jump-buckling load while  $P_{5,1}^U = P_5 = 0.0675$  is not.

Let us return to the unconstrained thick sphere experiments in which bifurcation buckling need not precede jump-buckling. As has been observed, only uniform contraction can occur for  $P < P_{6,1}^L$  which we therefore term the lower buckling load  $P^L$ . Since  $P_5$  is not a bifurcation buckling load we conclude that as  $P$  increases over the interval  $P^L \doteq 0.01045 < P < P_6 \doteq 0.07062$  the thick sphere can become nonspherical only by jumping into a buckled state. The multiplicity of solutions varies over this interval. For example, at  $P = 0.015$  there are eight such solutions corresponding to the branches\*:  $B_{6,0}$ ;  $B_{6,1}$ ;  $B_{5,1}^+$ ;  $B_{5,2}^+$ ;  $B_{5,7}^+$ . Of these solutions only those on  $B_{6,1}$  and  $B_{5,1}^+$  have more energy than the unbuckled state at  $P = 0.015$ . The states with negative energy are in some sense preferred to the unbuckled state. Jumping to a state of negative energy cannot occur for  $P < P_{6,0}^E$ . We must invoke some mechanism for the shell to jump into one of these states of negative energy. The usual such mechanism is attributed to

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\* As another example consider  $P = 0.01996$  where there are 16 solutions on the branches:  $B_{6,0}$ ;  $B_{6,1}$ ;  $B_{5,j}^+$ ;  $j = 1, 2, \dots, 7$ . All but the three solutions on  $B_{6,1}$  and  $B_{5,1}^+$  have less energy than the uniformly contracted sphere with  $P = 0.01996$ . In Figures 14 a,b we show 4 of these solutions.



imperfections and/or disturbances of the original spherical shell, of the loading procedure, of the shell support, etc. The transition is actually dynamic and since there are many buckled states present with energy less than the unbuckled state we do not know which state is the one finally preferred. The number and character of the buckled states which coexist at a given pressure may influence the possibility of jump-buckling. Thus the triggering mechanism which causes the shell to jump, no doubt, depends on the interaction of the imperfections, disturbances and the kind of buckled states that exist at a given pressure.

The possible buckling mechanism just described is such that precise jump-buckling loads cannot be defined. However, we note that for  $P$  just below a symmetric eigenvalue or an unsymmetric eigenvalue whose bifurcated solutions branch down there must always be buckled states very close to the unbuckled state but with slightly more energy. An example is  $P_5$  for the thick sphere. These states could also supply the triggering mechanism for jump buckling in experiments with very small imperfections or disturbances. In such a case the observed jump buckling load should be less than the lowest eigenvalue, that is,  $P_5$  for the thick sphere. Of course, if the imperfections are so small that they do not cause jump buckling, then as  $P$  increases bifurcation buckling must occur at  $P_n$  and finally jump buckling must occur at  $P_{n,1}^U$ .

We have only considered axisymmetric deformations in

this study. If we allow axially unsymmetric states there may be additional bifurcation branches and other buckled states. A numerical study of these additional bifurcation branches has not yet been initiated. However an analysis of the axially unsymmetric bifurcation branches near the eigenvalues and the effects of imperfections is given in [8] using a different shell theory.

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## Appendix I. Derivation of the Shell Theory.

The two dimensional Hooke's law for the normal stresses  $\sigma_\theta$  and  $\sigma_\phi$  is

$$(A.1) \quad \sigma_\theta = \frac{E}{1-\nu^2} (E_\theta + \nu E_\phi) , \quad \sigma_\phi = \frac{E}{1-\nu^2} (E_\phi + \nu E_\theta) .$$

Substituting (2.7) and (2.8) into (A.1) yields

$$(A.2a) \quad \sigma_\theta(r, \theta) = ES(\theta) - z\left(\frac{3ER}{h^2}\right)M , \quad \sigma_\phi(r, \theta) = ET(\theta) - z\left(\frac{3ER}{h^2}\right)N ,$$

where

$$(A.2b) \quad \begin{cases} S(\theta) = (1-\nu^2)^{-1}(e_\theta + \nu e_\phi) = (1-\nu^2)^{-1}[U' - W + \frac{1}{2} V^2 + \nu(U \cot \theta - W)] , \\ T(\theta) = (1-\nu^2)^{-1}(e_\phi + \nu e_\theta) = (1-\nu^2)^{-1}[U \cot \theta - W + \nu(U' - W + \frac{1}{2} V^2)] , \end{cases}$$

and

$$(A.2c) \quad \begin{cases} M(\theta) = \frac{Rk}{1-\nu^2} (\kappa_\theta + \nu \kappa_\phi) = \frac{k}{1-\nu^2} (V' + \nu V \cot \theta) , \\ N(\theta) = \frac{Rk}{1-\nu^2} (\kappa_\phi + \nu \kappa_\theta) = \frac{k}{1-\nu^2} (V \cot \theta + \nu V') . \end{cases}$$

The parameter  $k$  is defined in (2.9c). The dimensionless quantities  $S$  and  $T$  are proportional to  $\sigma_\theta$  and  $\sigma_\phi$  evaluated on the middle surface,  $z = 0$ . They are called the membrane stresses. The dimensionless quantities  $M$  and  $N$ , which we call the bending moments, are related to the physical bending moments  $M_\theta$  and  $M_\phi$  by

$$M = - M_\theta / (2EhR) , \quad N = - M_\phi / (2EhR) .$$

The strain energy of the shell is given approximately by

$$(A.3) \quad I \equiv \frac{1}{2} \int_{R-h}^{R+h} \int_0^{2\pi} \int_0^{\pi} (\sigma_{\theta} E_{\theta} + \sigma_{\phi} E_{\phi}) r^2 \sin \theta \, d\theta \, d\phi \, dr .$$

We insert (2.4), (2.7), (2.8), (A.1) and (A.2) into (A.3), perform the  $r$ ,  $\phi$  integrations and use  $h/R \ll 1$  to obtain

$$(A.4) \quad I = 2\pi R^2 E h \int_0^{\pi} [(S e_{\theta} + T e_{\phi}) + R(M \kappa_{\theta} + N \kappa_{\phi})] \sin \theta \, d\theta .$$

If  $p$  is a pressure then the work done,  $F$ , is defined by

$$(A.5) \quad F = -p\Delta$$

where  $\Delta$  is the change in the volume enclosed by the midsurface of the shell before and after deformation. The pressure is positive when it is directed inwards. To determine  $\Delta$  we first denote the projections on the equatorial plane and on the polar axis\* of the radius from the origin to any point on the shell's mid-surface respectively by  $\rho$  and  $\delta$ . Thus for the undeformed shell we have

$$(A.6) \quad \rho = \rho_u \equiv R \sin \theta, \quad \delta = \delta_u \equiv R \cos \theta ,$$

and for the deformed shell we have

$$(A.7) \quad \rho = \rho_d \equiv R \sin \theta + H, \quad \delta = \delta_d \equiv R \cos \theta - J .$$

Here  $H$  and  $J$ , the horizontal and vertical components of the displacement of a point on the shell's mid-surface, are given by

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\* The polar axis is the diameter through the north and south poles. We assume the shell is oriented so that the polar axis is vertical. The equatorial plane, defined by  $\theta = \pi/2$ ,  $0 \leq \phi \leq 2\pi$ , is thus horizontal.

$$(A.8) \quad \frac{H}{R} = (U \cot \theta - W) \sin \theta = e_{\phi} \sin \theta, \quad \frac{J}{R} = W \cos \theta + U \sin \theta.$$

J is positive downwards.  $\Delta$  is defined by

$$(A.9) \quad \Delta \equiv \pi \int_0^{\pi} (\rho_u^2 \delta_u' - \rho_d^2 \delta_d') d\theta.$$

We substitute (A.6)-(A.8) into (A.9) and use (2.8a) to get

$$(A.10) \quad \Delta = \pi R^3 \int_0^{\pi} [e_{\phi}^3 + 3e_{\phi} + 3e_{\phi}^2] + (U' + W' \cot \theta)(1 + e_{\phi})^2 \sin^3 \theta d\theta.$$

Since  $|e_{\phi}| \ll 1$ ,  $\Delta$  is given approximately by

$$(A.11) \quad \Delta = \pi R^3 \int_0^{\pi} [U' + W' \cot \theta + 3e_{\phi}] \sin^3 \theta d\theta.$$

Employing the expression for  $e_{\phi}$  given by (2.8a) in (A.11), we obtain after integration by parts,

$$(A.12) \quad \Delta = -2\pi R^3 \int_0^{\pi} W \sin \theta d\theta.$$

Thus from (A.5) and (A.12) the work done by the normal pressure is

$$(A.13) \quad F = 2\pi R^3 p \int_0^{\pi} W \sin \theta d\theta.$$

We observe that (A.13) is also the work done by a uniformly distributed surface force  $p$  which is directed toward  $r = 0$ .

Hence, for this shell theory, normal pressure and a centrally directed surface load have, within terms of higher order, the same work expression. Consequently the boundary value problems will be the same for both loads.

The potential energy  $G$  is defined as

$$(A.14) \quad G \equiv I - F ,$$

where  $I$  and  $F$  are given by (A.4) and (A.13). Employing (2.6), (2.8) and (A.2b,c) in (A.14), we express  $G$  as a functional of  $u$  and  $w$ . The Euler equations of this functional are

$$(A.15a) \quad Q \sin \theta + (S \sin \theta)' - T \cos \theta = 0 ,$$

$$(A.15b) \quad (Q \sin \theta)' - (S+T)\sin \theta - 2P \sin \theta = 0 ,$$

where the shear  $Q$  is defined by

$$(A.15c) \quad Q = [(M \sin \theta)' - N \cos \theta - SV \sin \theta] \csc \theta ,$$

and  $P$  is defined in (2.9c). Since the solutions are to be bounded and axisymmetric with respect to the polar axis the boundary conditions at  $\theta = 0, \pi$  are

$$(A.16) \quad U = W' = Q = 0 .$$

We observe that

$$(A.17) \quad U = V = M = N = Q = 0, \quad S = T = -P, \quad W = W_0 \equiv (1-\nu)P ,$$

is a solution, for all  $P$  and  $k$ , of the system of equations (A.2), (A.15), (2.6) and satisfies the boundary conditions (A.16). It represents the uniformly contracted sphere or the unbuckled state. We seek other solutions in the form



$$(A.18) \quad W = W_0 + w(\theta) , \quad S = -P + s(\theta) , \quad T = -P + t(\theta) ,$$

$$U = u(\theta) , \quad V = v(\theta) , \quad \dots , \quad Q = q(\theta) .$$

We substitute (A.18) into (A.2), (A.15), (2.6) and (A.16) and find that the lower case variables must satisfy

$$(A.19a) \quad q \sin \theta + (s \sin \theta)' - t \cos \theta = 0 ,$$

$$(A.19b) \quad (q \sin \theta)' - (s+t)\sin \theta = 0 ,$$

$$(A.19c) \quad (m \sin \theta)' - n \cos \theta - sv \sin \theta - q \sin \theta + Pv \sin \theta = 0 ,$$

$$(A.19d) \quad v = u + w' ,$$

$$(A.19e) \quad \begin{cases} s = \left( \frac{1}{1-v^2} \right) [u' - w + \frac{1}{2} v^2 + v(u \cot \theta - w)] , \\ t = \left( \frac{1}{1-v^2} \right) [u \cot \theta - w + v(u' - w + \frac{1}{2} v^2)] , \end{cases}$$

$$(A.19f) \quad \begin{cases} m = \frac{1}{1-v^2} (v' + vv \cot \theta) , \\ n = \frac{1}{1-v^2} (v \cot \theta + vv') , \end{cases}$$

$$(A.19g) \quad u = w' = q = 0 , \quad \theta = 0, \pi .$$

Equations (A.19a-f) are a system of eight equations in eight dependent variables. We eliminate the four variables  $s$ ,  $n$ ,  $u$  and  $w$  from this system and obtain the four ordinary differential equations (2.9a,b). The boundary conditions (2.14) follow from (A.19d,g). When a solution of this boundary value problem is determined we evaluate the eliminated variables from

$$(A.20) \quad s = q \cot \theta , \quad n = vm + kv \cot \theta .$$

The equations to evaluate  $u$  and  $w$  are obtained by first eliminating  $w$  from (A.19e). This yields the first order ordinary differential equation for  $u$ , (2.18).

If  $G_0$  is the potential energy of the unbuckled state (A.17), then the relative potential energy  $e$  of a buckled state is

$$(A.21) \quad e \equiv G - G_0 .$$

We insert (A.4) and (A.13) in (A.14) and express  $G$  in terms of  $m$ ,  $q$ ,  $t$ ,  $v$  and  $w$ . Integration by parts and (A.19g) gives (2.19).



Figure 1a

Graphs of  $P$  vs. the amplitude  $A$ , which is defined in Equation (5.1), corresponding to the 7 lowest symmetric eigenvalues and the 4 lowest unsymmetric eigenvalues for the thick sphere  $k = .001$ . The circles on the  $P$  axis indicate the symmetric eigenvalues and the crosses the unsymmetric eigenvalues. Several of the lower symmetric branches  $B_{n,j}$  and the lower unsymmetric branches  $B_{n,j}^{\pm}$  are explicitly labeled in the graph. Several of the upper  $P_{n,j}^U$  and lower  $P_{n,j}^L$  critical loads are also shown. The branches  $B_{5,j}^{\pm}$ ,  $j = 1, 2, \dots, 7$  and  $B_{6,1}$  and  $B_{6,0}$  are not completely shown in the figure since the corresponding values of  $A$  are too large. They are shown in Figure 1b. The circles on the graphs indicate the pressures at which unsymmetric branches merge with symmetric branches.

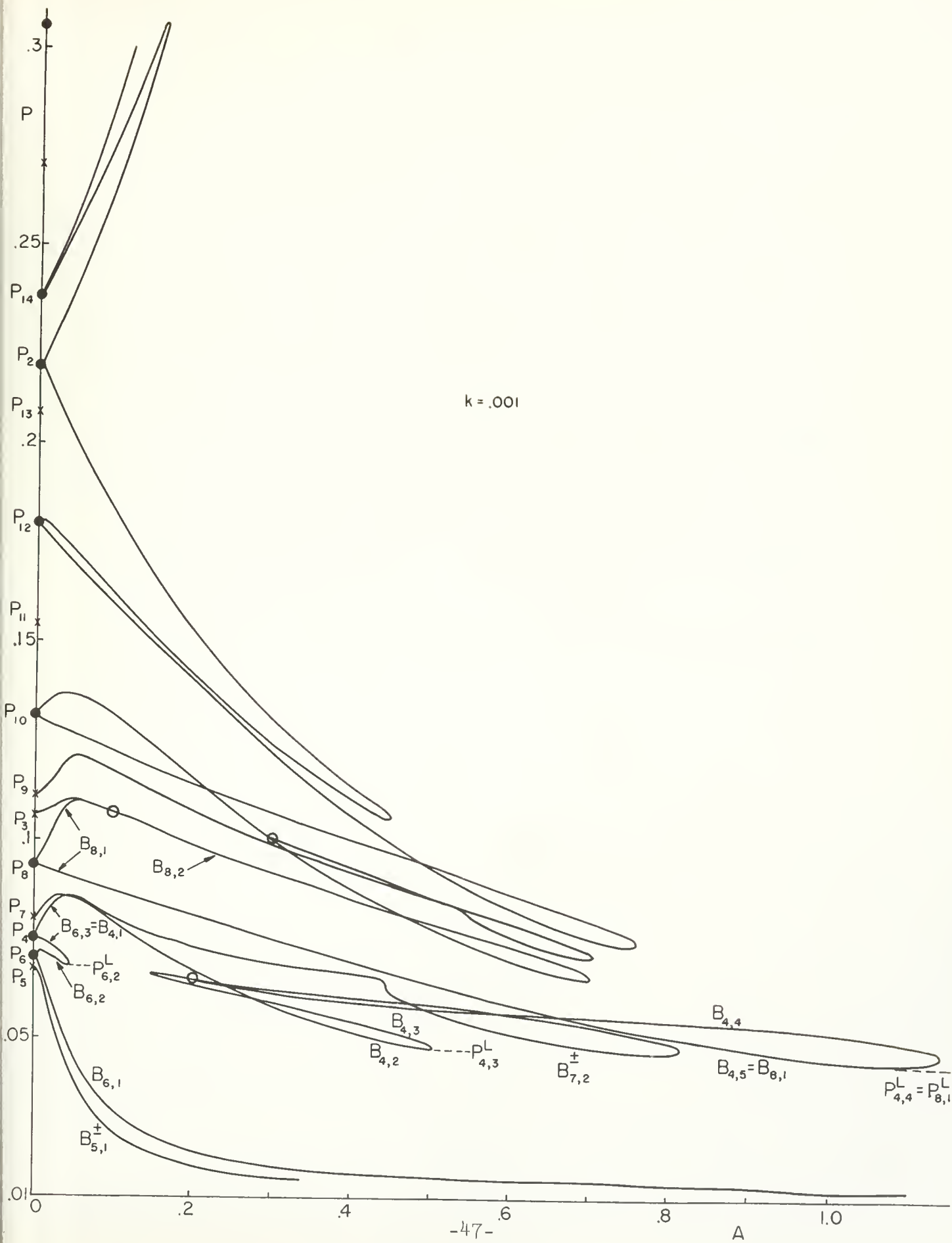


Figure 1b

Graphs of  $P$  vs.  $A$  for  $B_{6,1}$  with  $P \leq P_6$ ,  $B_{6,0}$  and  $B_{5,j}^{\pm}$ ,  $j = 1, 2, \dots, 7$ . The scale of the graph is insufficient to resolve the branches  $B_{5,j}^{\pm}$ ,  $j = 2, \dots, 7$  completely. The unsymmetric branches  $B_{5,7}^{\pm}$  merge with the symmetric branch  $B_{6,1}$  at  $P = P_{5,7}^L \doteq 0.0124$ . The circle on the graph indicates the pressure at which  $B_{5,7}^{\pm}$  merge with  $B_{6,1}$ .

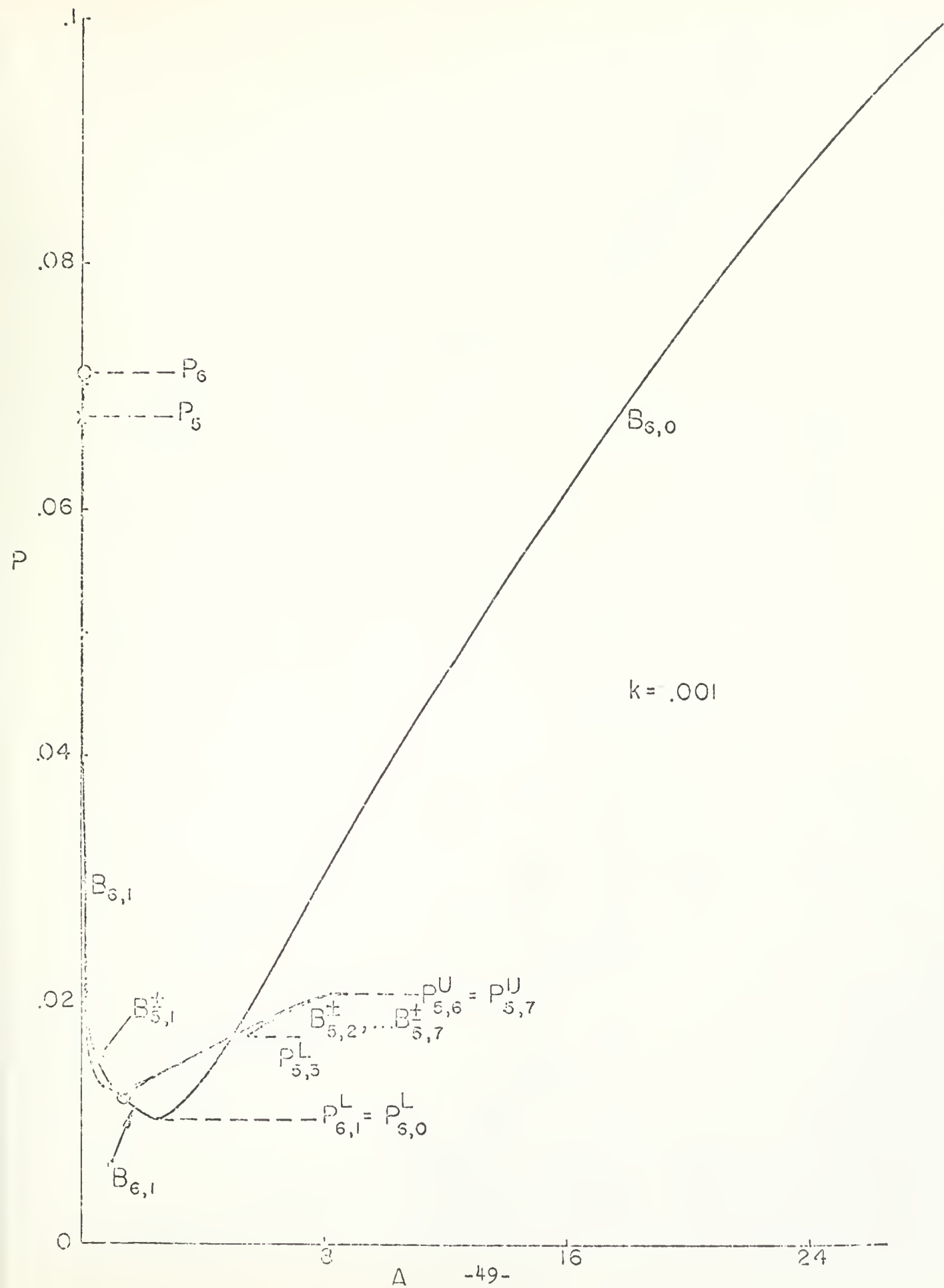


Figure 2a

Graphs of  $(e/2)^{1/3}$  vs.  $P$  for the thick sphere,  $k = .001$ . The  $P$  axis is horizontal. The circles on the  $P$  axis indicate the symmetric eigenvalues and the crosses the unsymmetric eigenvalues. Some of the critical loads and branches are labeled. The circles on the graphs indicate the pressures at which unsymmetric branches merge with symmetric branches. The equal energy loads  $P_{n,j}^E$  occur where the curves cross the  $P$  axis at points other than eigenvalues. The  $P_{n,j}^E$  are too close to  $P_n$  to be clearly shown on  $B_{n,j}$  with  $n \geq 12$ . The branches  $B_{6,0}$  and  $B_{5,j}$ ,  $j = 2, 3, \dots, 7$  are not completely shown in this figure since the values of  $-(e/2)^{1/3}$  are too large. They are shown in Figure 2b.



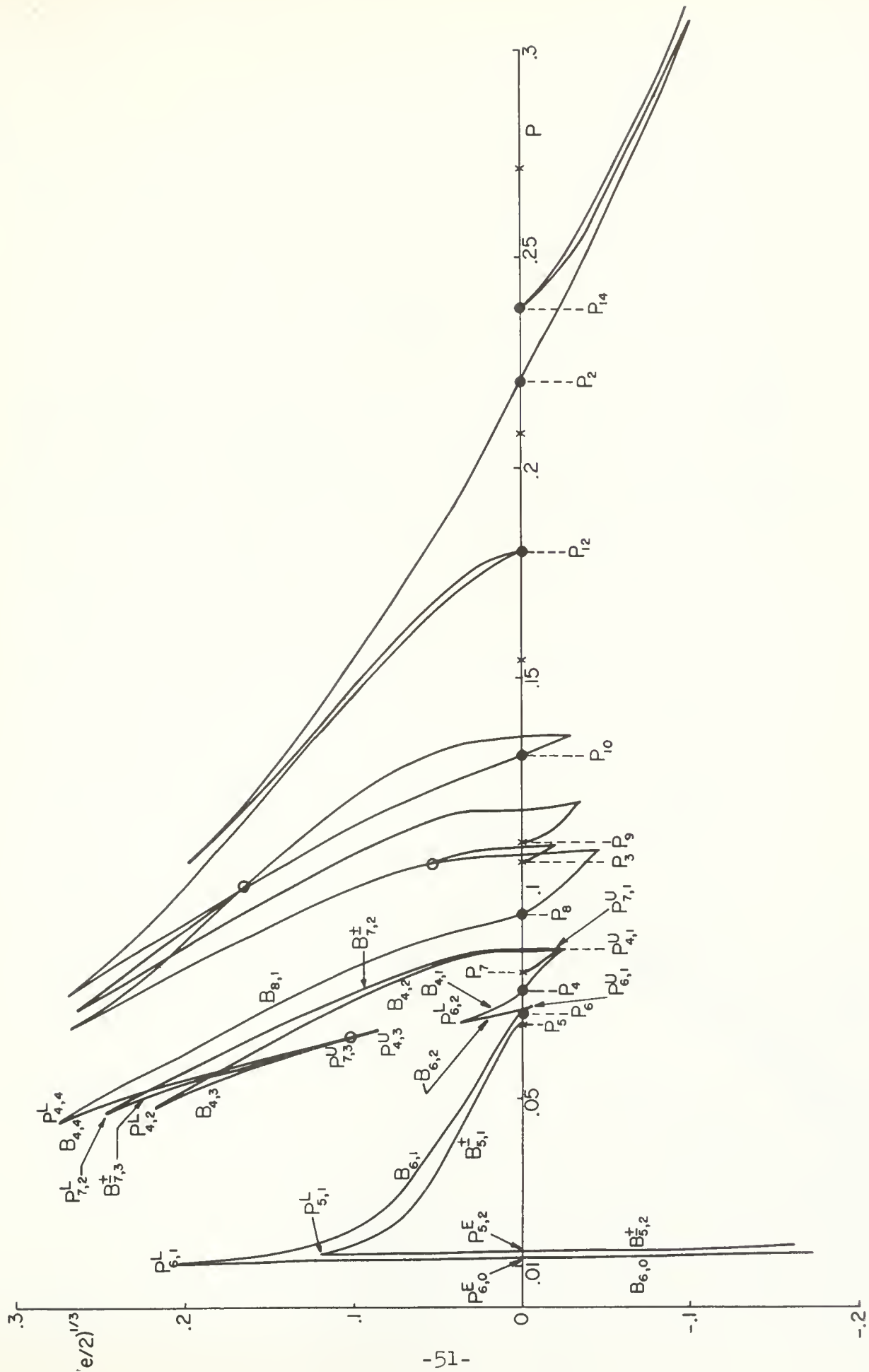


Figure 2b

Graphs of  $(e/2)^{1/3}$  vs.  $P$  for the lower part of the branches bifurcating from the lowest two eigenvalues  $P_5$  and  $P_6$ . The eigenvalues are not shown on the graph since their magnitudes exceed the scale. The branch  $B_{6,0}$  has been continued as far as  $P = .1$ , where  $(e/2)^{1/3} \doteq -1.4$ .  $N(P)$  denote the number of buckled solutions for  $P$  in the interval  $\underline{P} < P < \bar{P}$ . The following table shows how  $N$  varies for the range of  $P$  given in the figure.

$\bar{P}$	$P_{6,1}^L$	$P_{5,7}^L$	$P_{5,1}^L$	$P_{5,3}^L$	$P_{5,5}^L$	$P_{5,4}^U$	$P_{5,2}^U$	$P_{5,6}^U$
$\underline{P}$	0	$P_{6,1}^L$	$P_{5,7}^L$	$P_{5,1}^L$	$P_{5,3}^L$	$P_{5,5}^L$	$P_{5,4}^U$	$P_{5,2}^U$
$N$	0	2	4	8	12	16	12	8

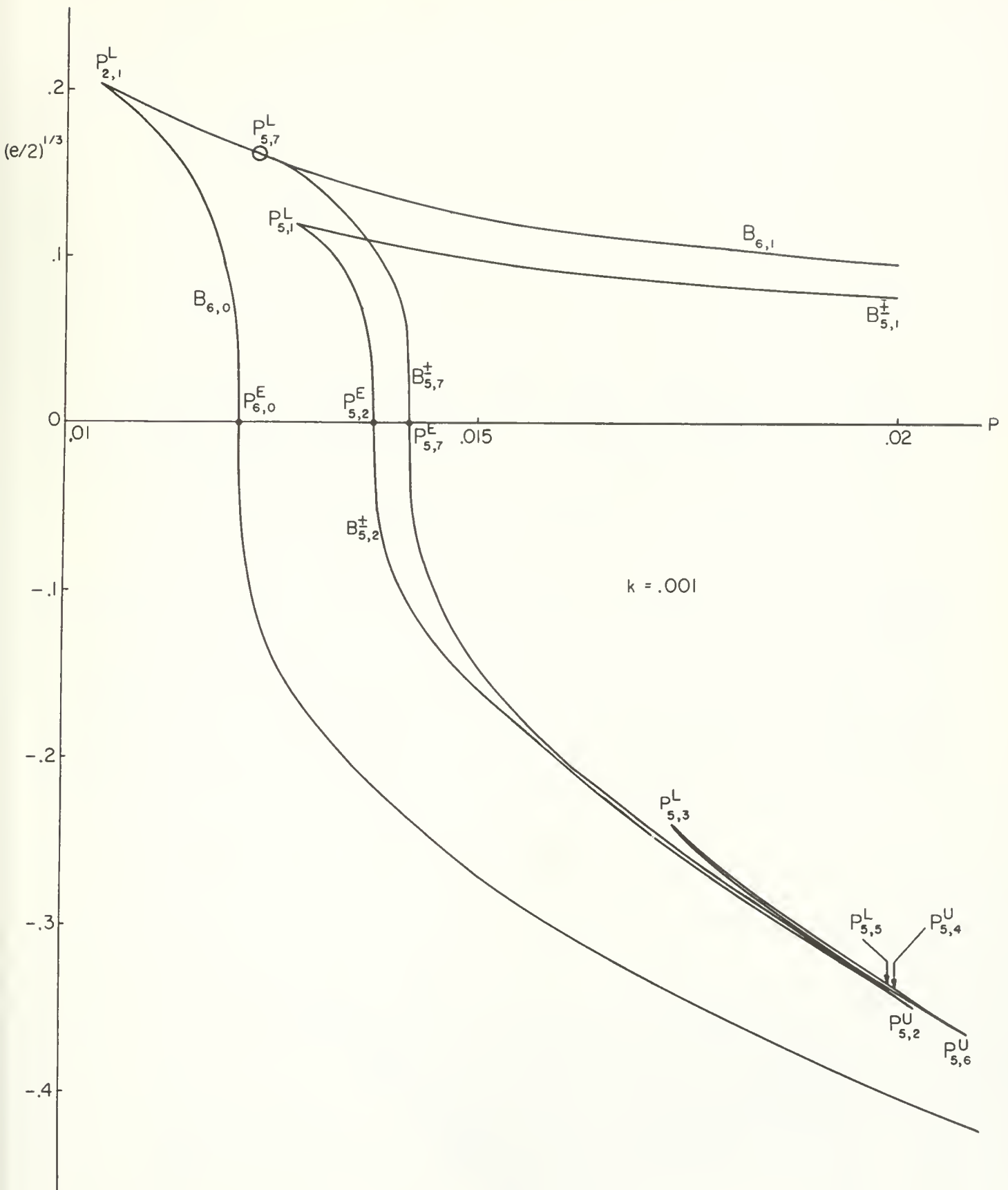
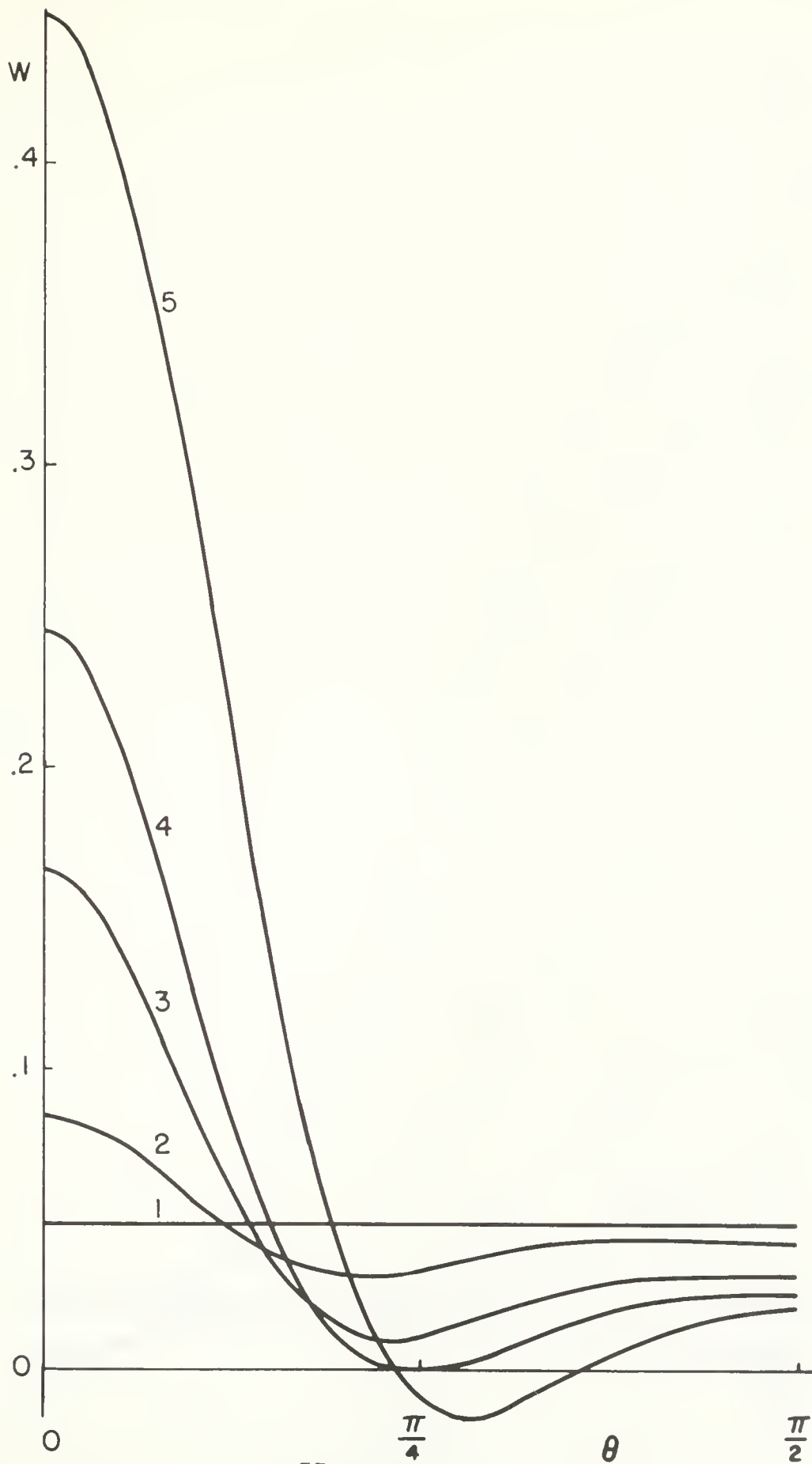
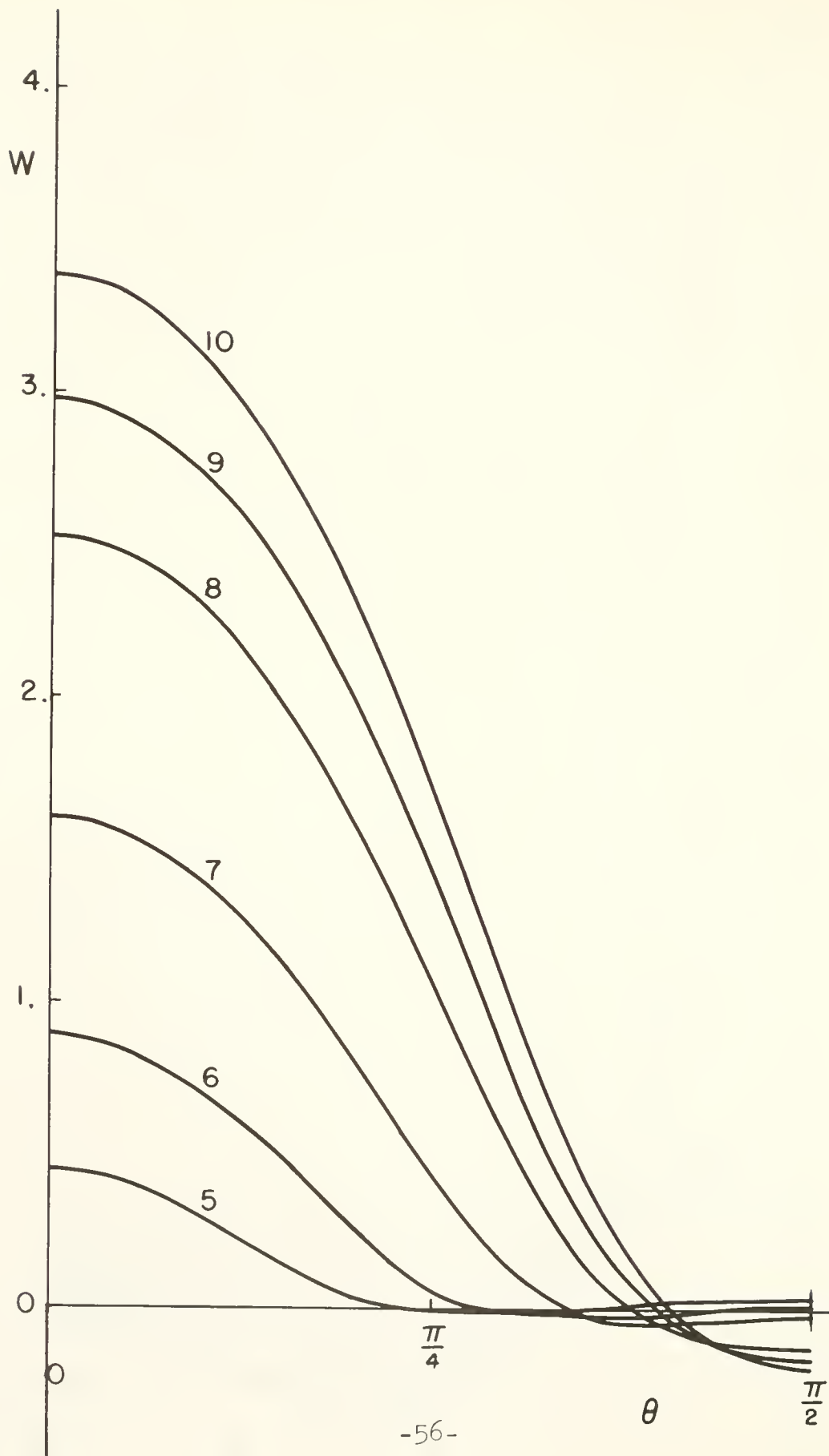


Figure 3

Graphs of the radial displacement  $W(\theta) \equiv w(\theta) + W_0$ , where  $W_0$  is the uniform radial contraction of the unbuckled state. The solutions are for a sequence of values of  $P$  as indicated in the following table. This table also applies to Figure 4.

Graph Number	1	2	3	4	5	6	7	8	9	10	11	12	13	
P	$P_6$	.06	.04	.03	.02	.015	.013	.011	.0105	.0105	.03	.075	.1	
Branch	$B_{6,1}$	$\longleftrightarrow$							$B_{6,1}$	$B_{6,0}$	$\longleftrightarrow$			$B_{6,0}$





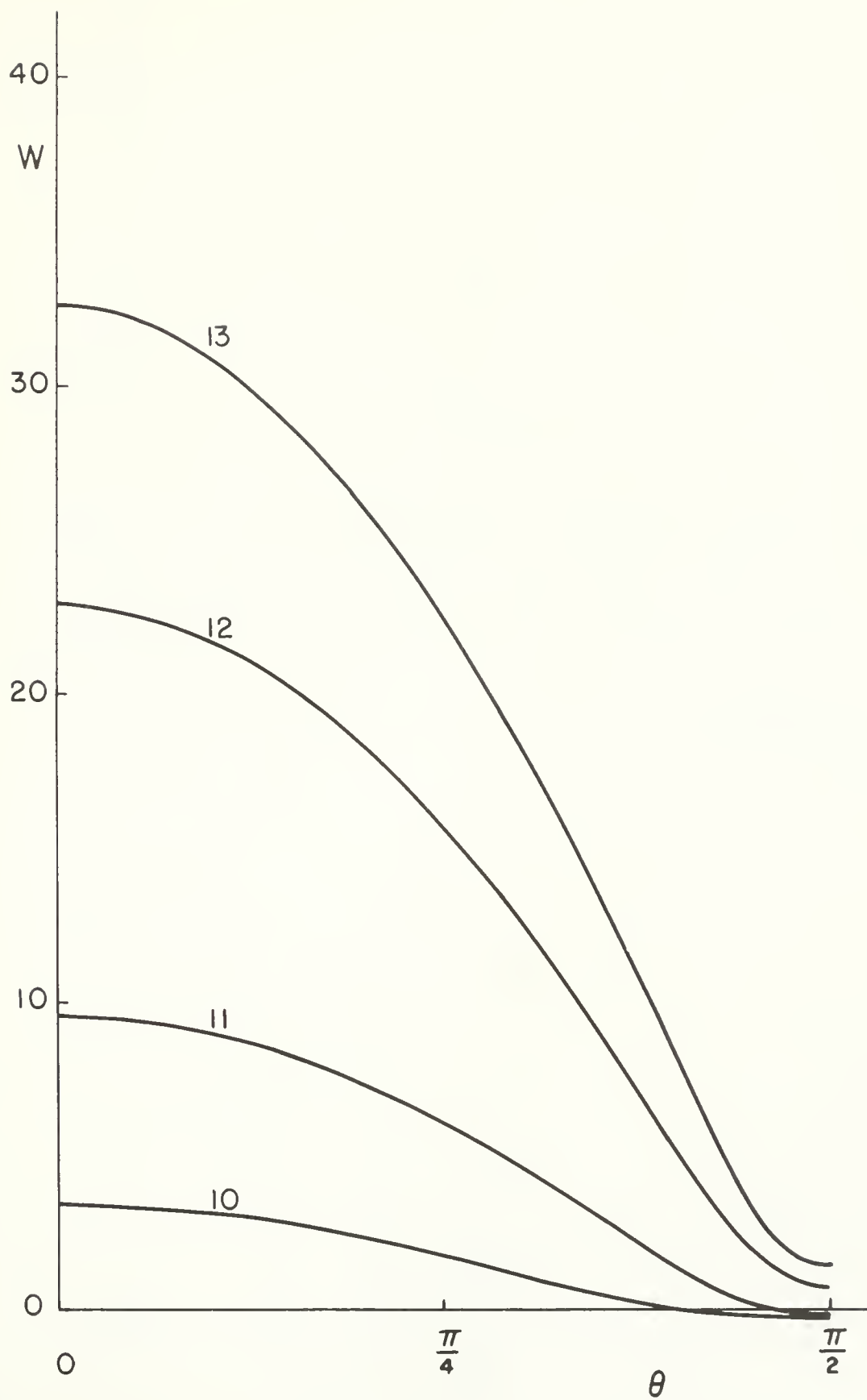
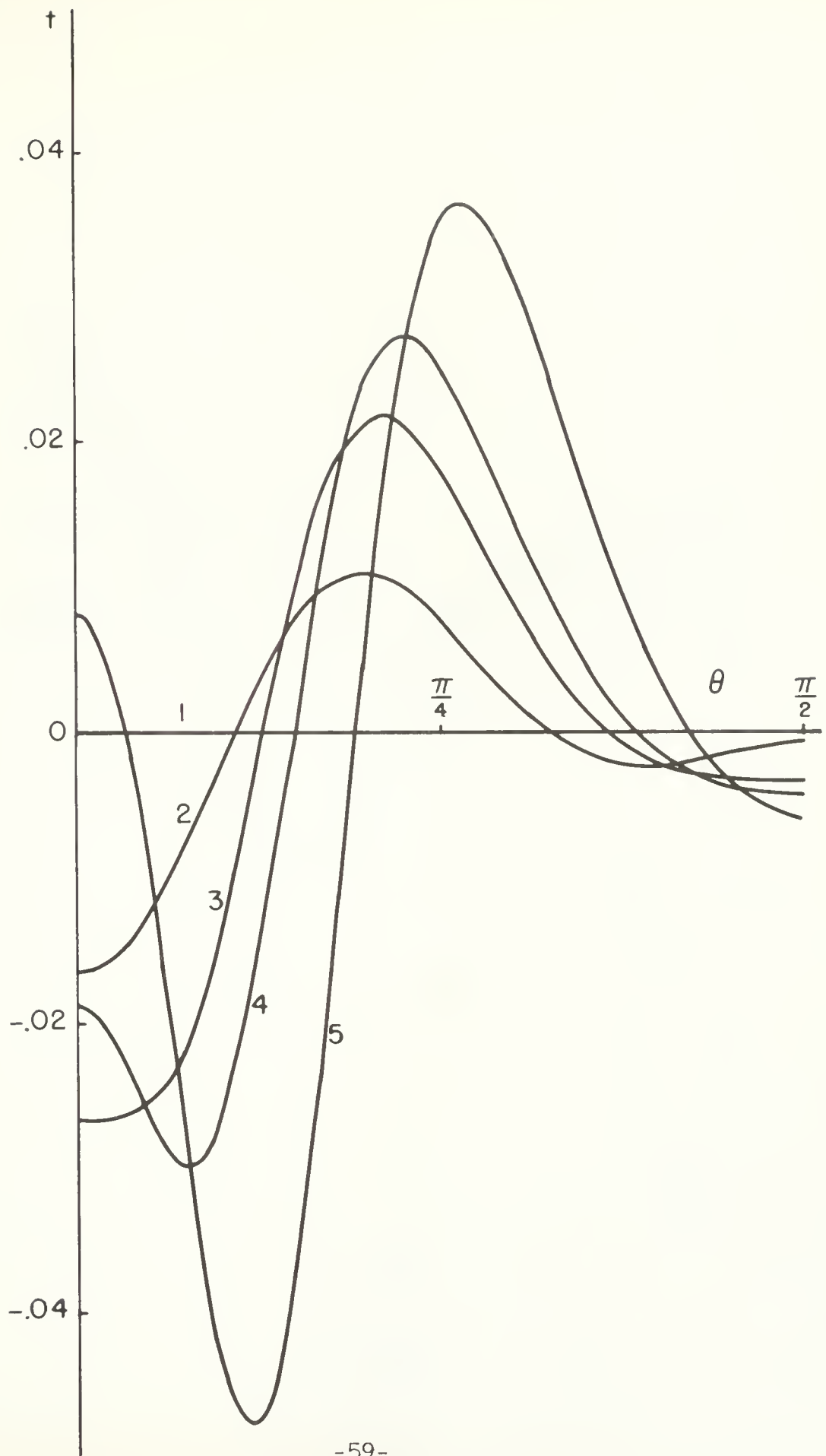
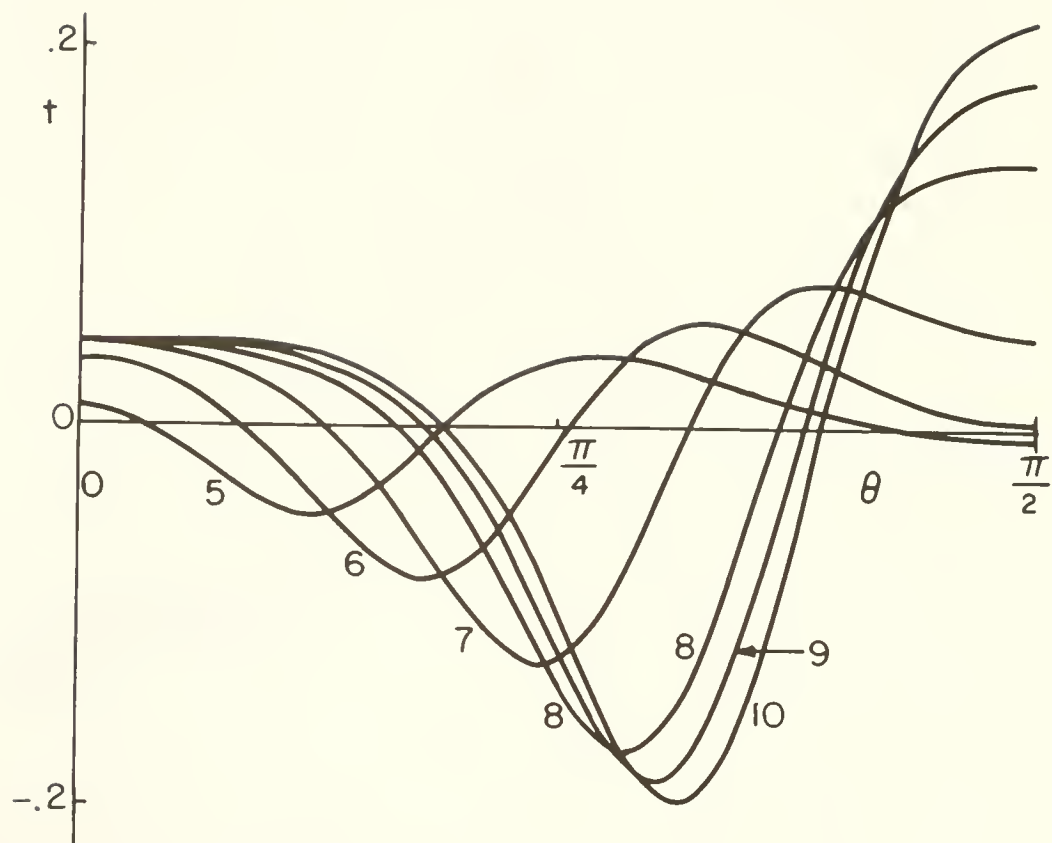


Figure 4

Graphs of the reduced circumferential stress  $t(\theta)$ , see Eq. (A.18). The solutions are for the sequence of  $P$  values given in the table in the caption for Figure 3.







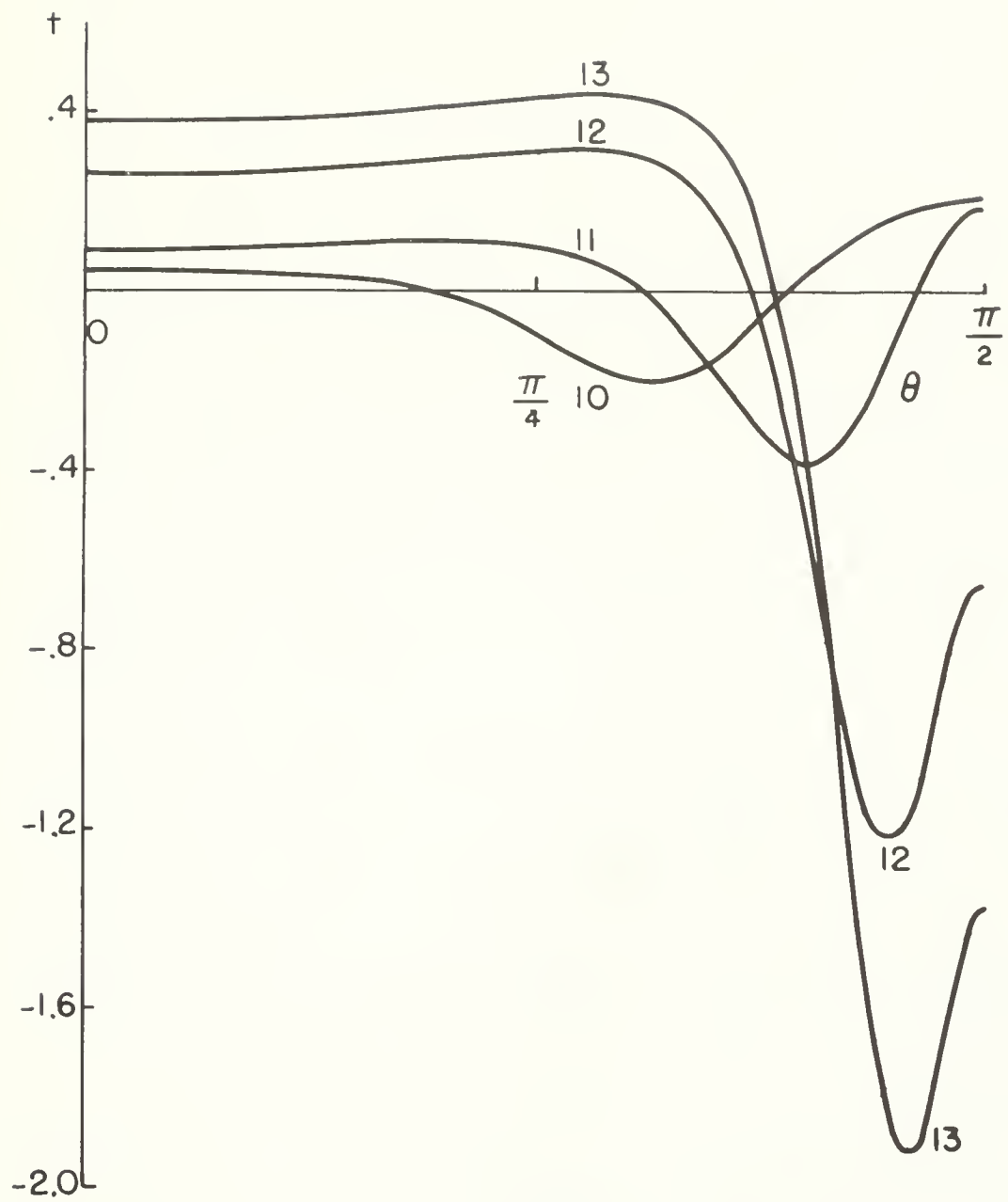


Figure 5

Graphs of  $W(\theta)$  for a sequence of values of  $P$  to show the transition in buckled shapes from  $B_{6,1}$  to  $B_{6,2}$  to  $B_{4,1}$ . The graphs numbered 1 and 4 are for  $P$  near the eigenvalues  $P_6$  and  $P_4$ . The graphs numbered 2 and 3 are for  $P$  near the critical loads  $P_{6,1}^U$  and  $P_{6,2}^L$ .

Graph Number	1	2	3	4
$P$	.070	.0712594 $\doteq P_{6,1}^U = P_{6,2}^U$	0.678672 $\doteq P_{6,2}^L = P_{4,1}^L$	.074
Branch	$B_{6,1}$	$B_{6,1}$ or $B_{6,2}$	$B_{6,2}$ or $B_{4,1}$	$B_{4,1}$

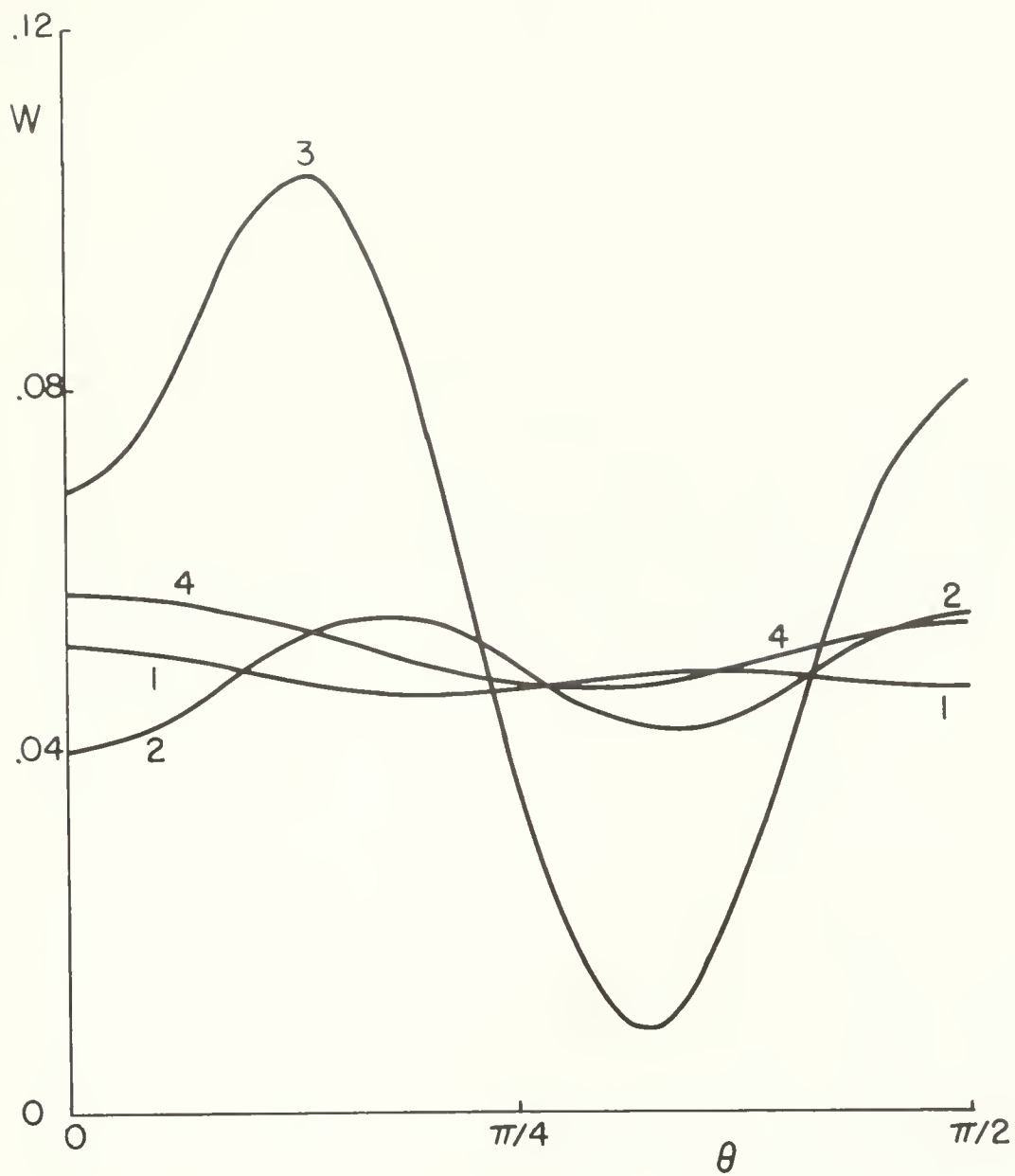


Figure 6

Graphs of  $t(\theta)$  for the same sequence of values of  $P$  as given in the table of the caption for Figure 5.

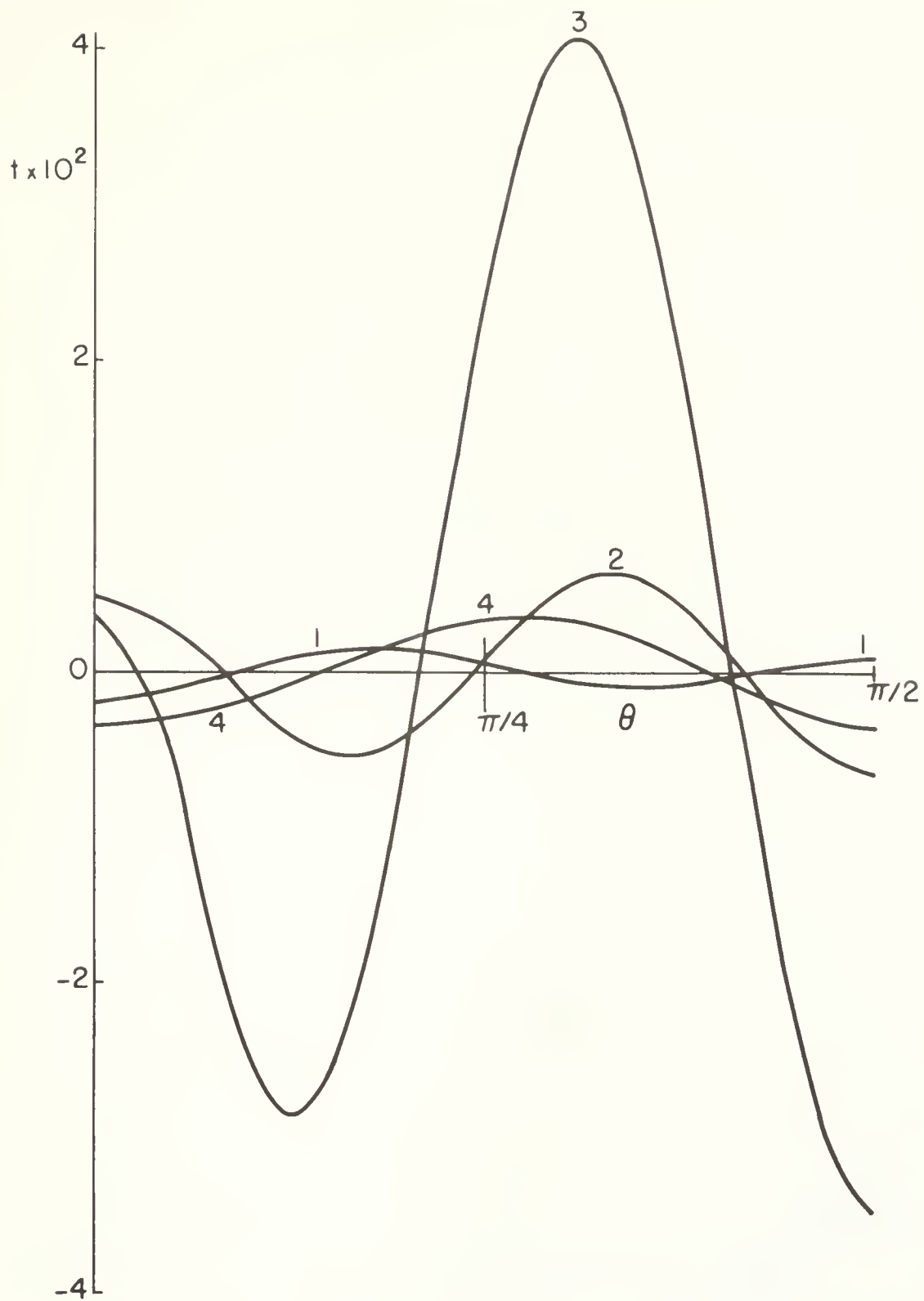


Figure 7a

Graphs of  $W(\theta)$  corresponding to points on  $B_{5,1}^+$  for a sequence of decreasing  $P$  values. Graphs 1, 2 and 3 are respectively for  $P = .066, .061, .040$ .



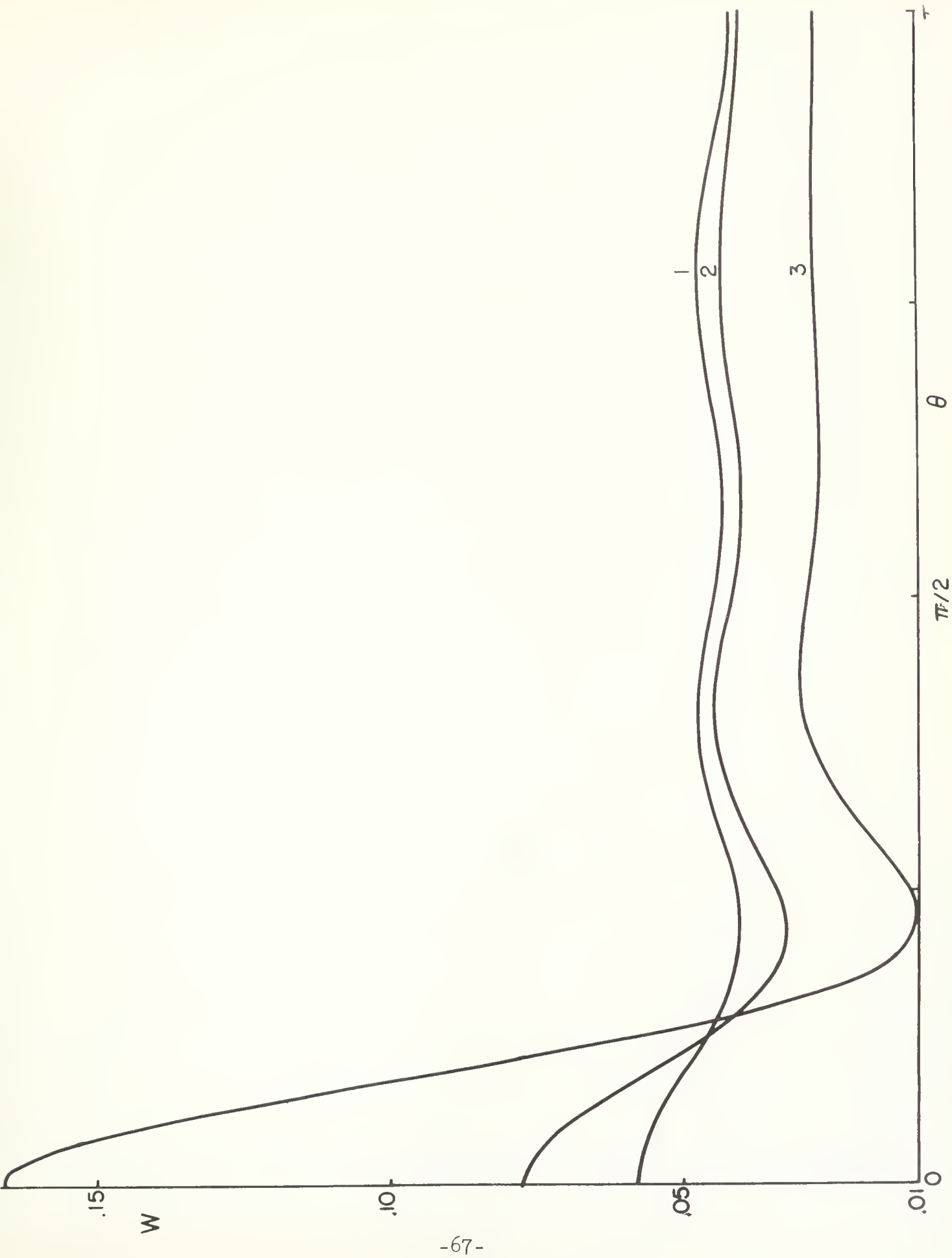


Figure 7b

Graphs of  $W(\theta)$  for a sequence of values of  $P$  on  $B_{5,j}^+$  with  $j$  increasing. The purpose of the figure is to show how the unsymmetric solutions gradually merge with the symmetric solution at  $P = p_{5,7}^L \doteq .012397$ . Graphs 4, 5, 6 and 7 are respectively for  $P = .0128125 \doteq p_{5,1}^L = p_{5,2}^L$ ,  $P = .0173426 \doteq p_{5,3}^L = p_{5,4}^L$ ,  $P = .020854 \doteq p_{5,6}^U = p_{5,7}^U$ , and  $P = .013$ . The curve 7 is on  $B_{5,7}^+$  near  $p_{5,7}^L$  where the unsymmetric and symmetric solutions merge.

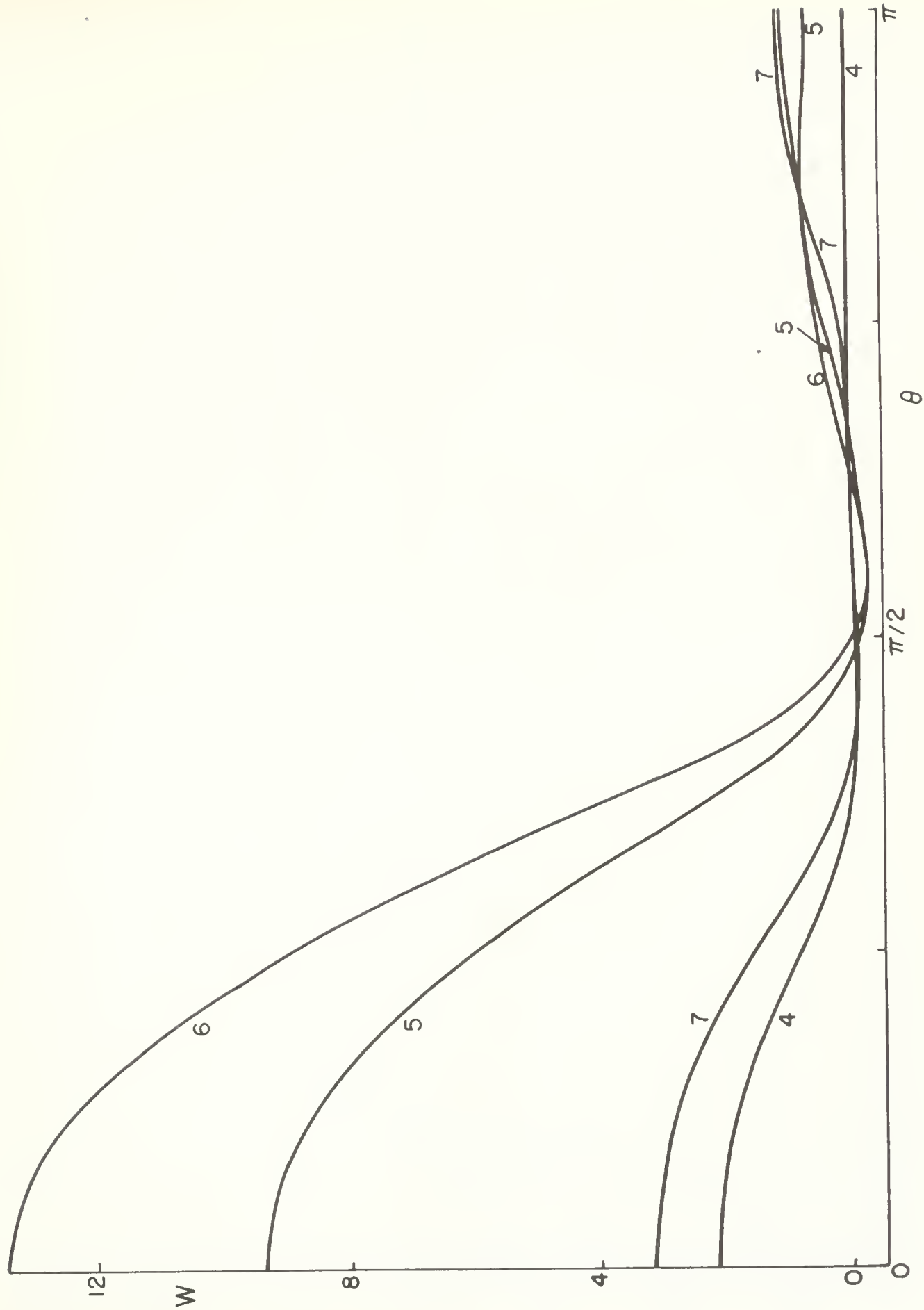


Figure 8a

Graphs of  $t(\theta)$  corresponding to points on  $B_{5,1}^+$  for the same values of  $P$  as in Figure 7a.

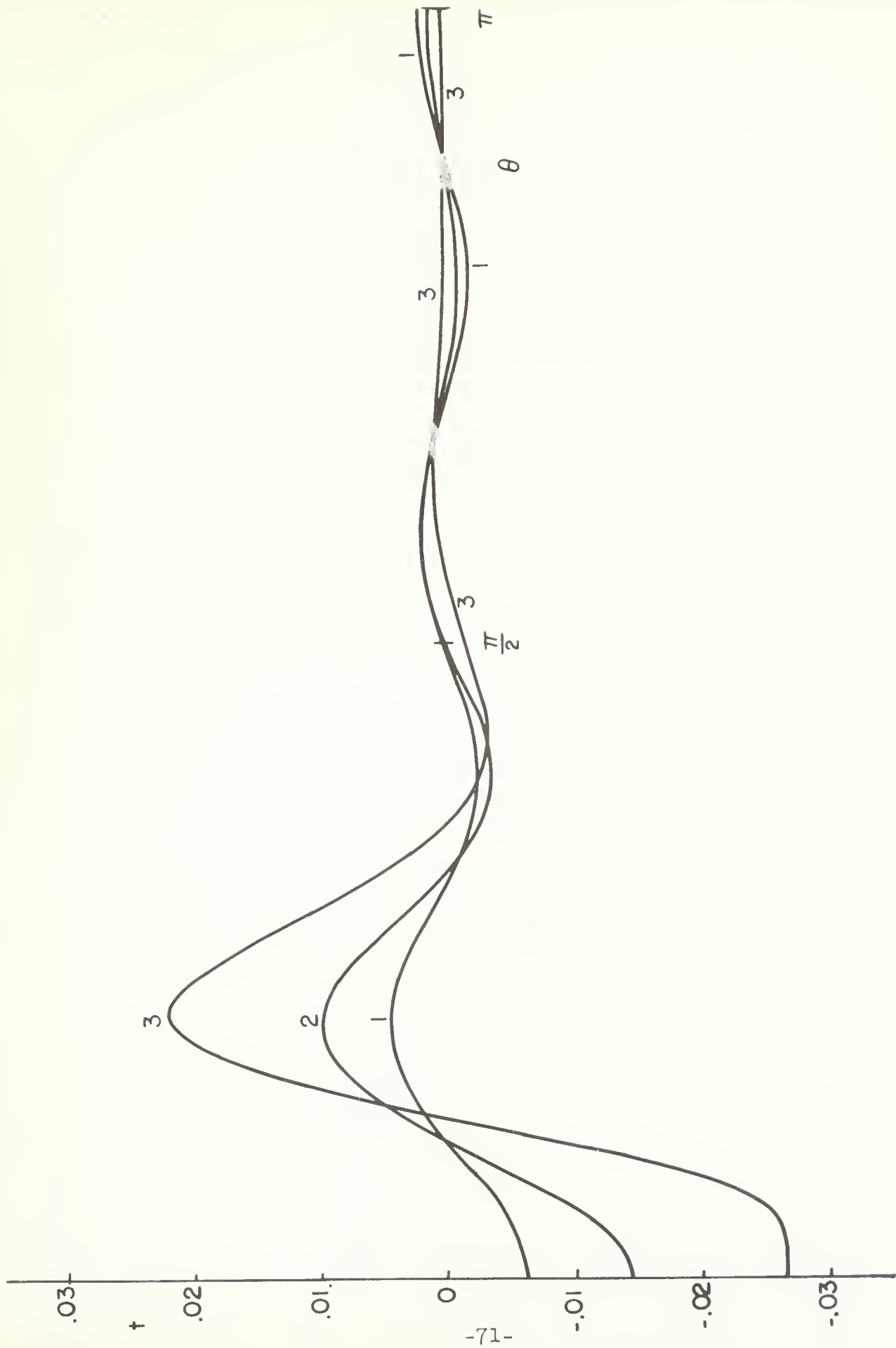


Figure 8b

Graphs of  $t(\theta)$  corresponding to the same solutions and pressures as in Figure 7b and in addition Graph 8 for  $P = .012399$ . Since  $P = .012399$  is close to  $P_{5,7}^L \doteq .0123971$  this solution is nearly symmetric.

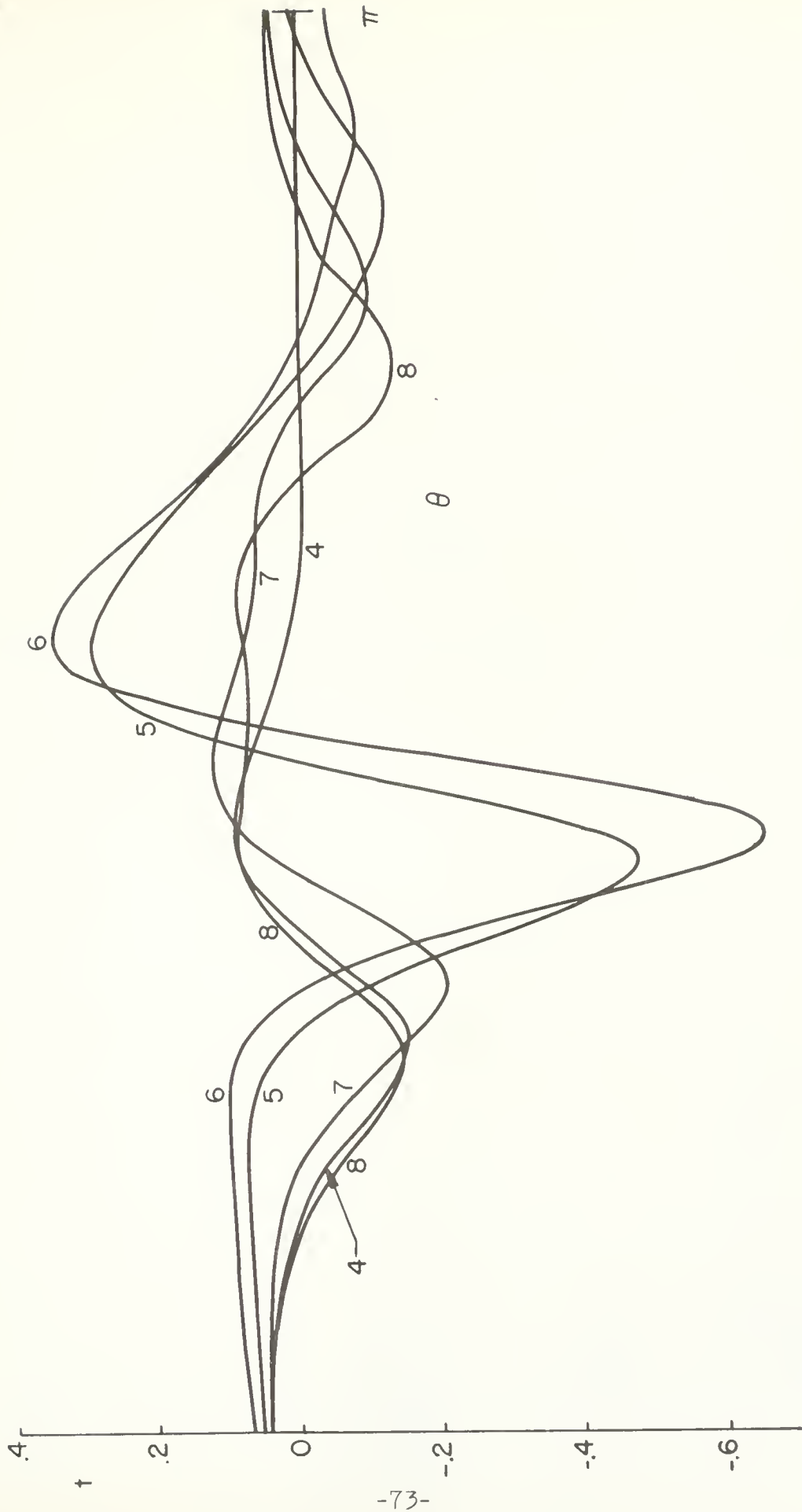


Figure 9

Graph of  $P$  vs.  $A$  for the solution branching from  $P_{16}$ , the lowest symmetric eigenvalue of the thin sphere.





Figure 10a

Graphs of  $(e/2)^{1/3}$  vs.  $P$  for the thin sphere,  $k = 10^{-5}$ , for the solutions branching from the lowest 5 symmetric eigenvalues. As in Figure 2a, the  $P$  axis is horizontal and the circles on the  $P$  axis indicate the eigenvalues. Some of the branches and critical loads are labeled. The branch  $B_{16,1}$  is not completely shown in the figure. It is shown in Figure 10b.

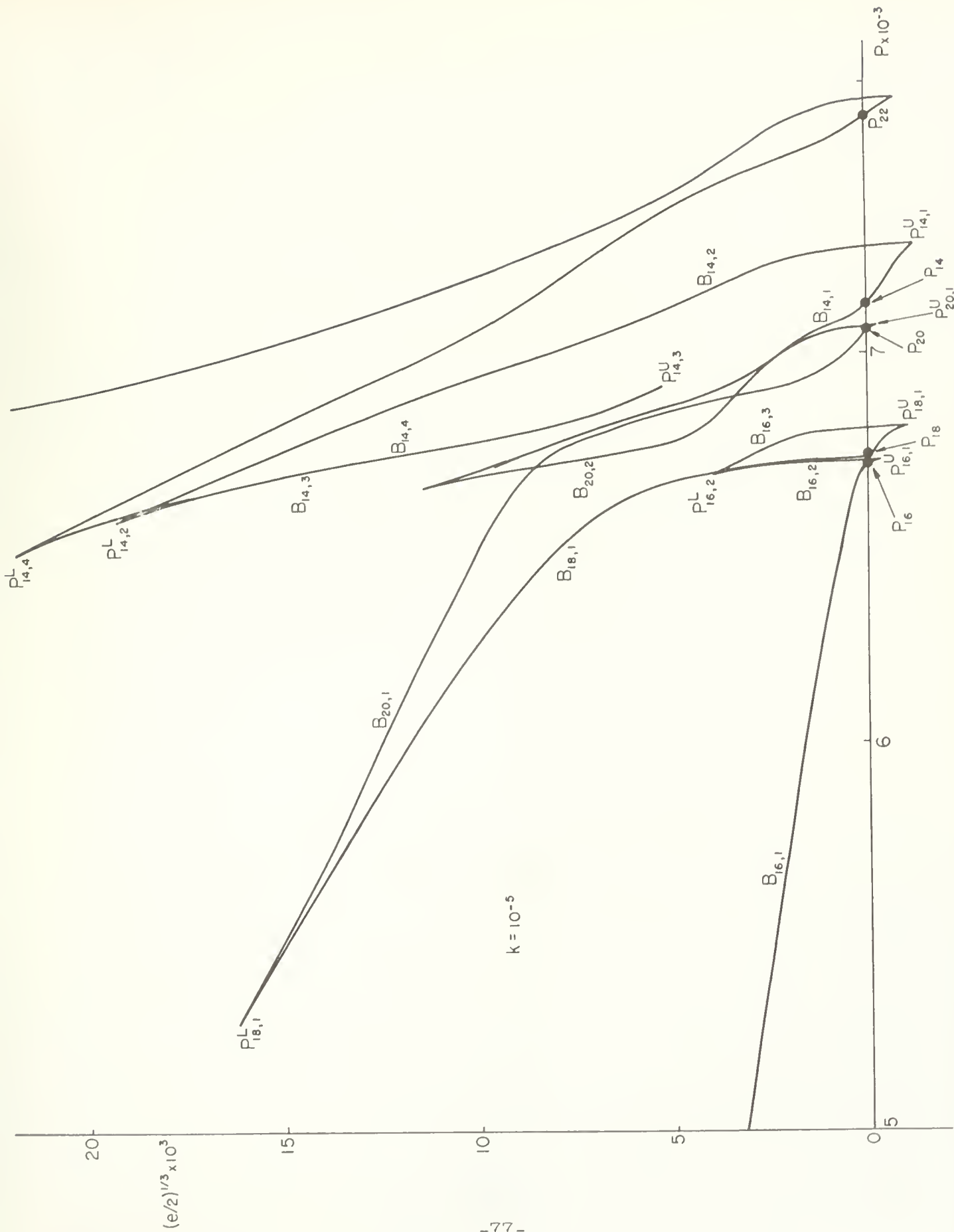


Figure 10b

Graphs of  $(e/2)^{1/3}$  vs.  $P$  for the lower part of  $B_{16,1}$  and for the branches  $B_{16,0}$ ,  $B_{16,-1}$  and a part of  $B_{16,-2}$ .

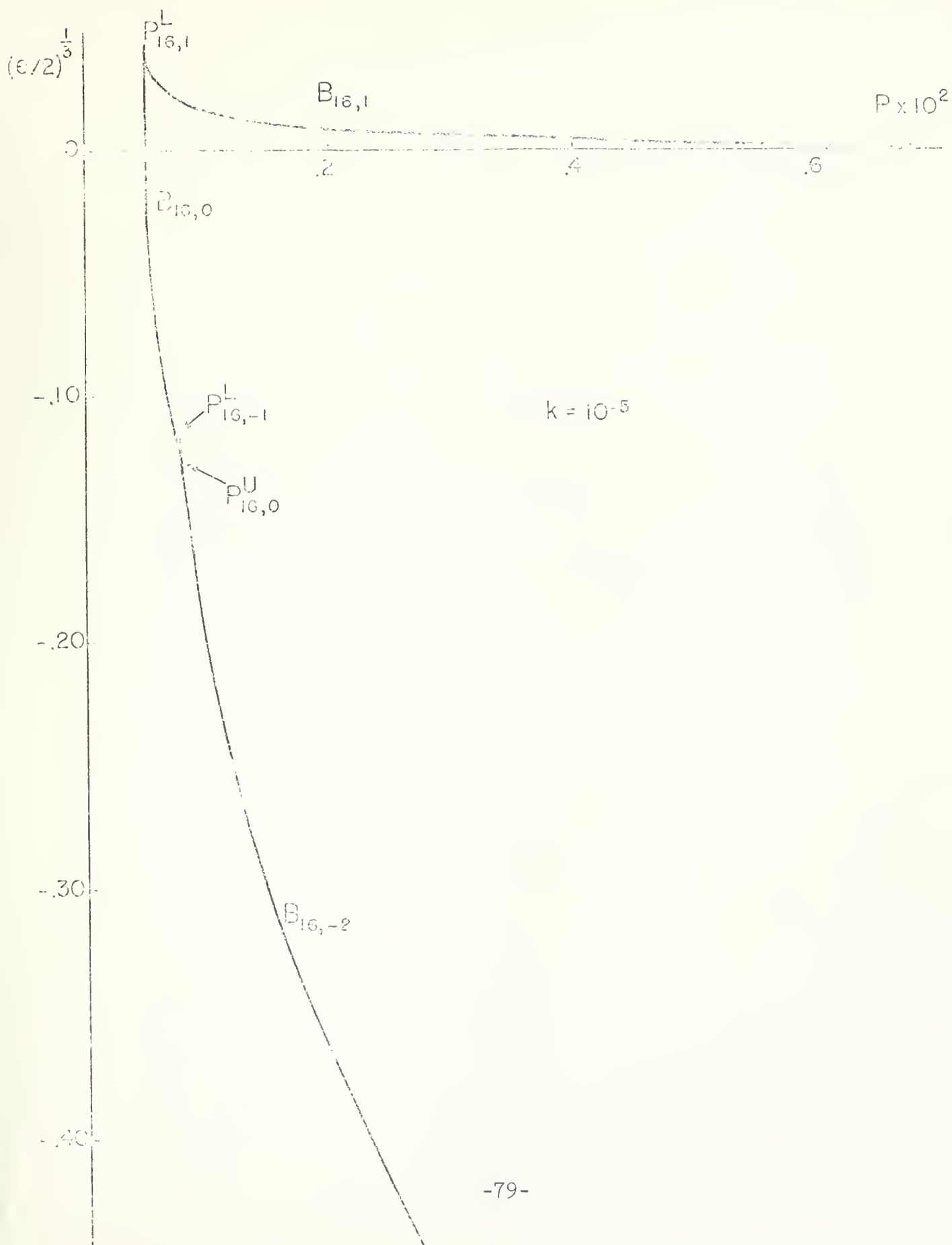


Figure 11

A sketch indicating the local behavior near the lowest eigenvalues for the sphere with  $k = 1.2 \times 10^{-5}$ . The numerical values for the critical pressures are  $P_{16} \doteq .0073217$ ,  $P_{16,1}^U = P_{16,2}^U \doteq .0073567$ ,  $P_{16,2}^E \doteq .0073552$ ,  $P_{17} \doteq .0073640$ ,  $P_{15} \doteq .007392$ .

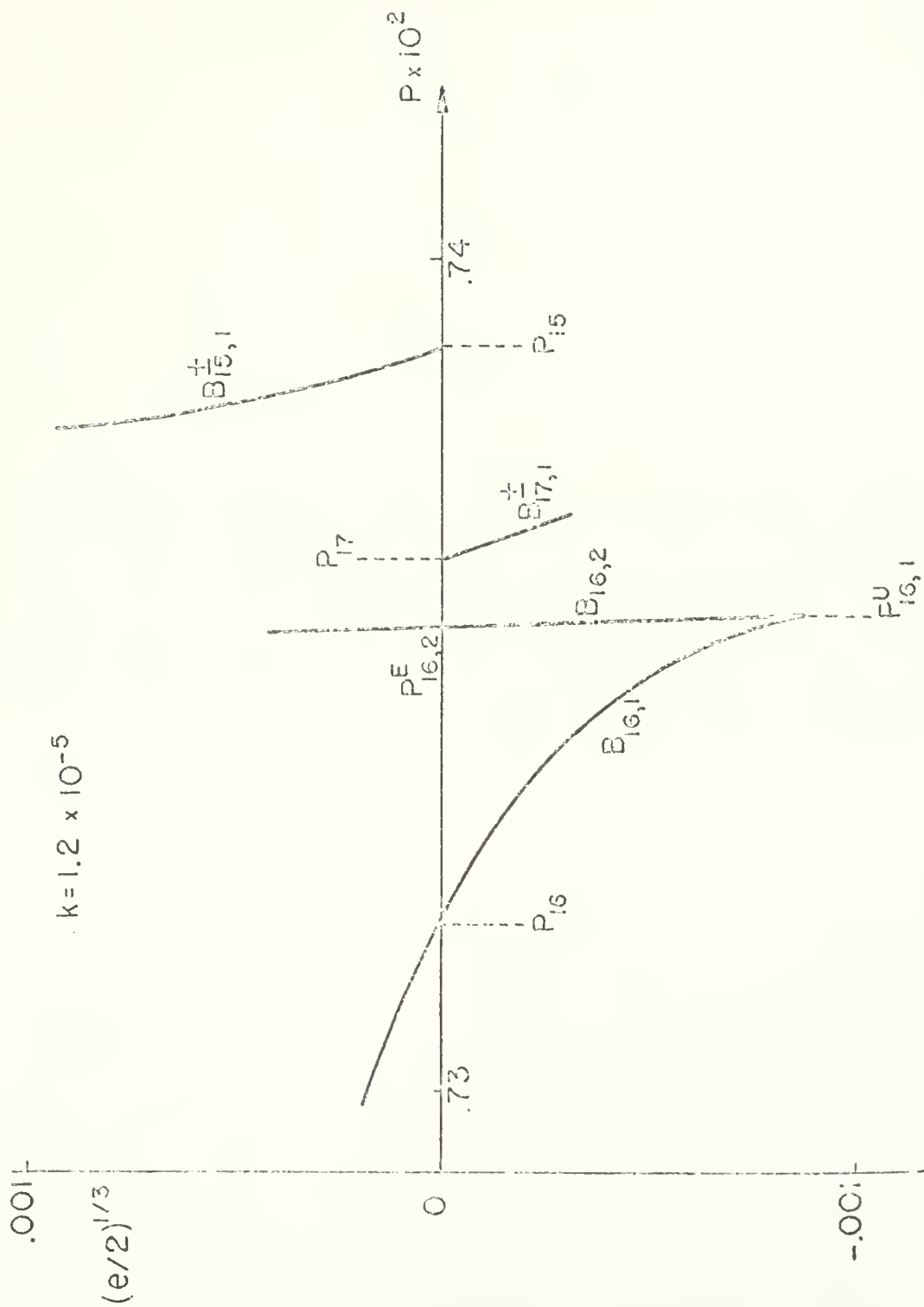


Figure 12

Graphs of  $W(\theta)$  for  $P = 6 \times 10^{-4}$  and a sequence of values of  $k$  decreasing from  $k = 10^{-5}$  (the thin sphere) to  $k = 2 \times 10^{-6}$ . For  $k = 10^{-5}$  the solution corresponds to a point on  $B_{16,1}$ . The values of  $k \times 10^5$  are 1, .8, .6, .4, and .2 for the graphs numbered 1 through 5 in that order.



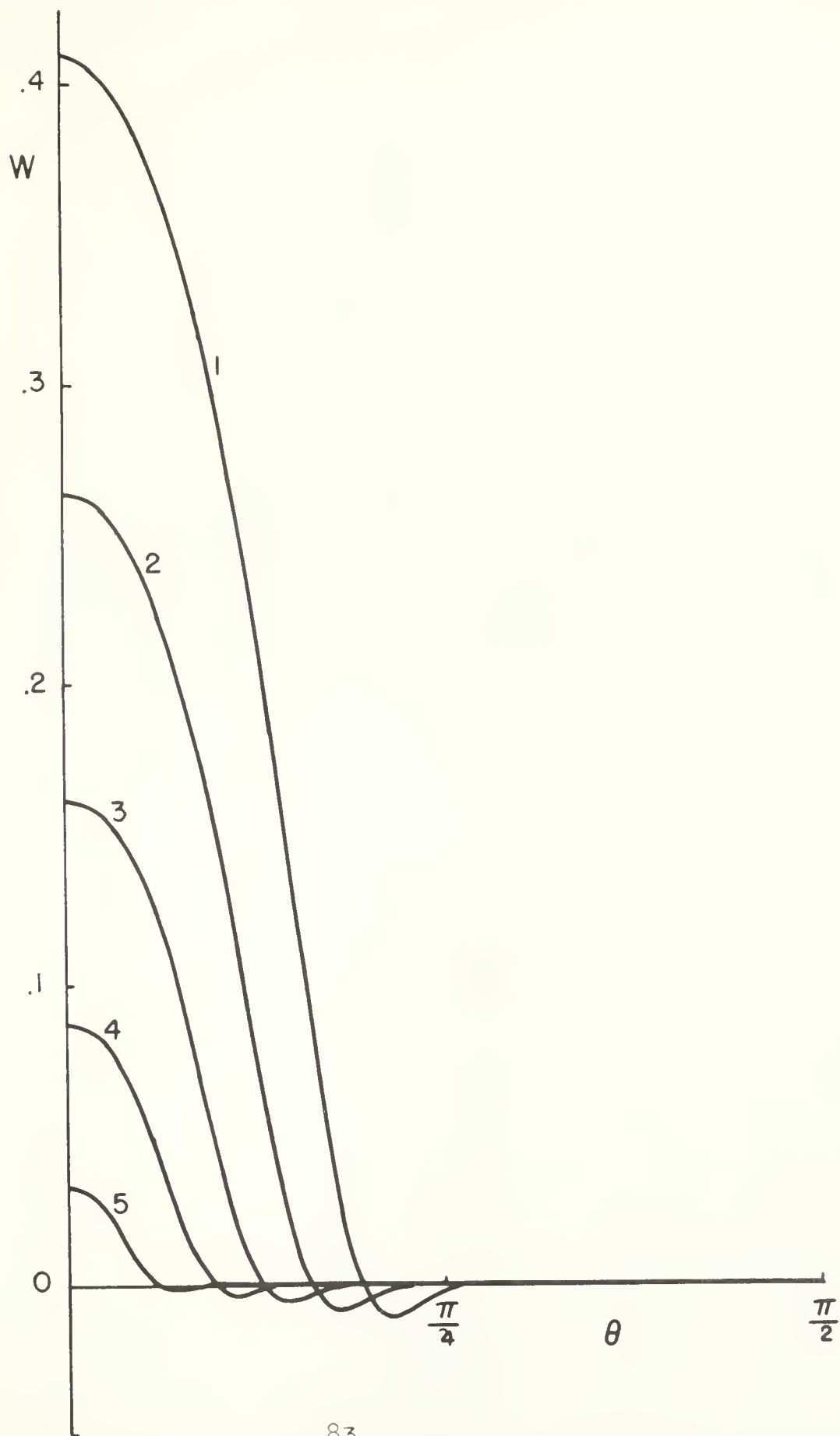


Figure 13

Graphs of  $t(\theta)$  for  $P = 6 \times 10^{-4}$  and the same sequence of values of  $k$  as in Figure 12.

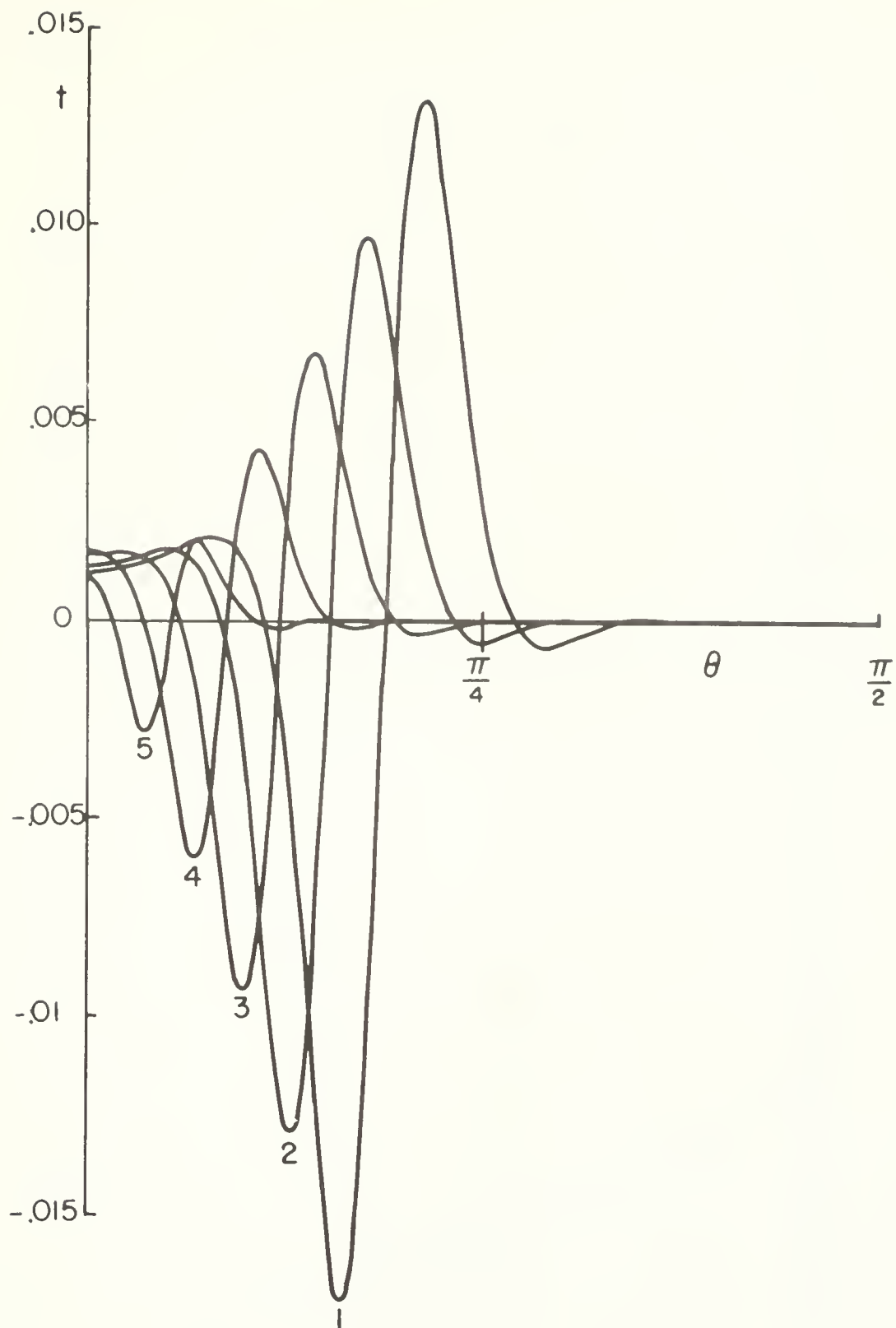
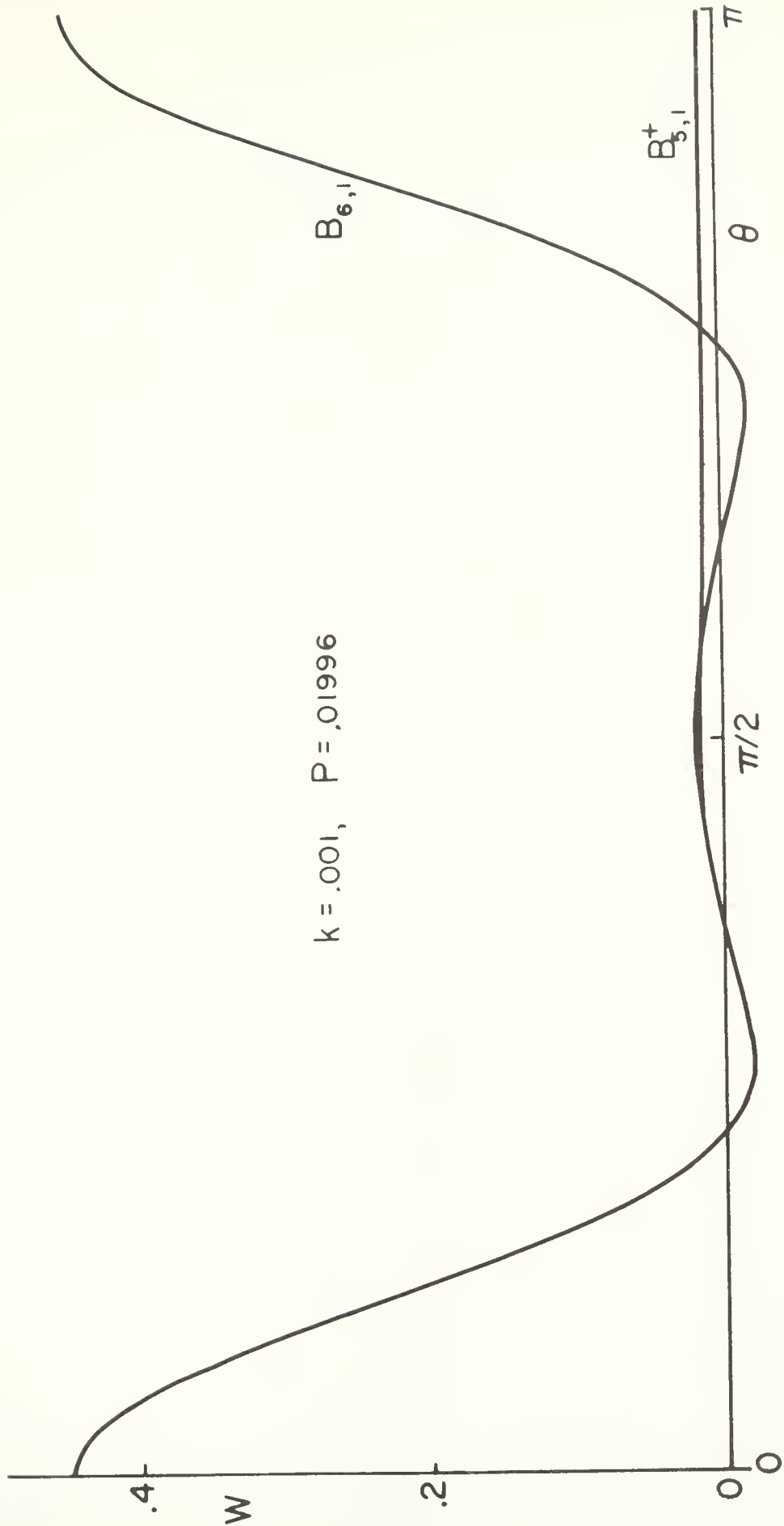


Figure 14a

Graphs of  $W(\theta)$  for two of the 16 solutions at  $P = .01996$  for the thick sphere. Two other solutions are shown in Figure 14b. The remaining solutions are not shown since they differ little from the first solution shown in Figure 14b.

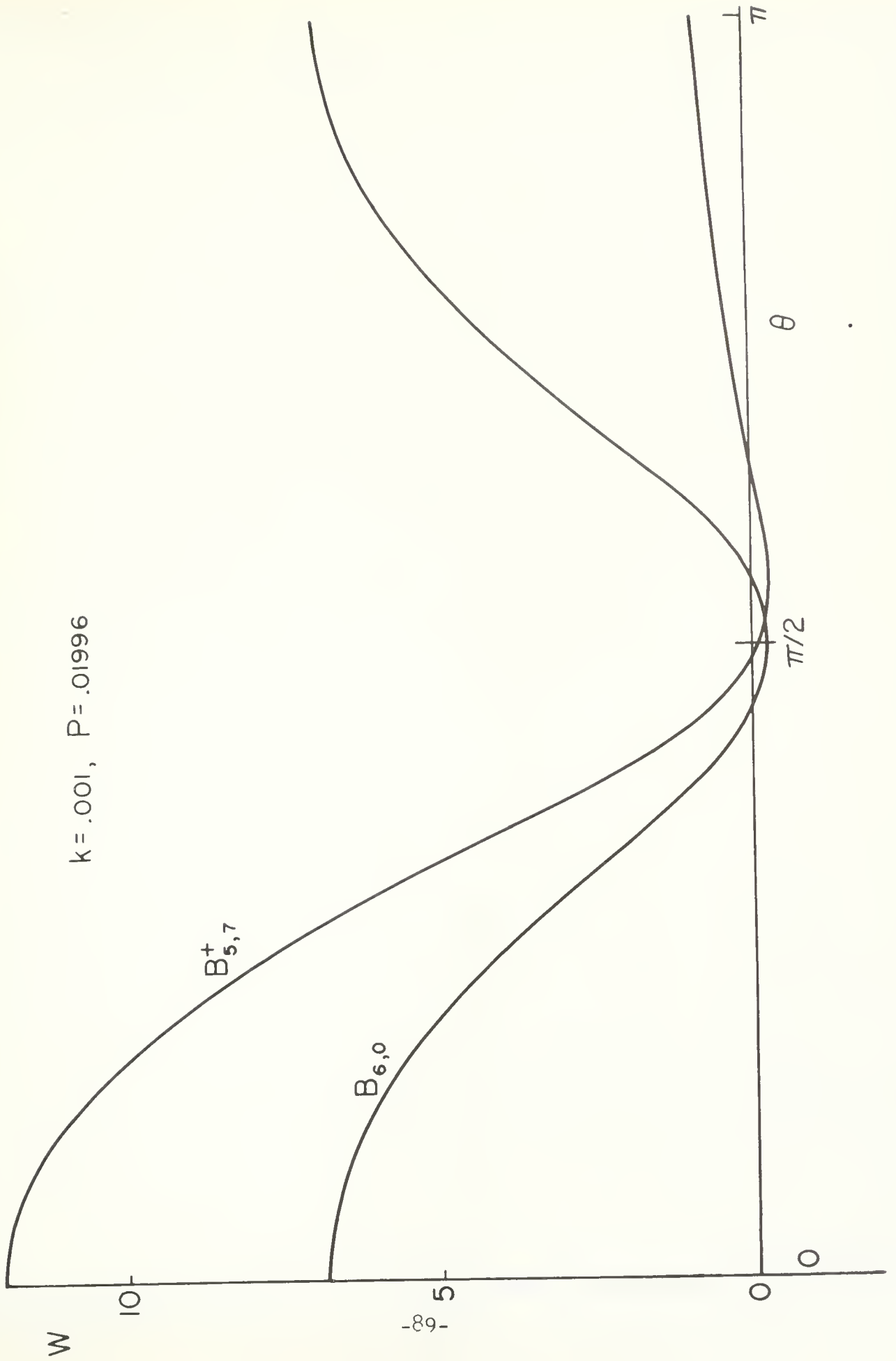


$$k = .001, \quad P = .01996$$

Figure 14b

Graphs of  $W(\theta)$  for two of the 16 solutions at  $P = .01996$ .

$k = .001, P = .01996$







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13. ABSTRACT A nonlinear thin shell theory is derived for the axisymmetric buckling of spherical shells subjected to either a pressure or a centrally directed surface load. The theory is reduced to a boundary value problem for a system of four first order ordinary differential equations. Numerical solutions of this boundary value problem are obtained by the shooting and parallel shooting methods. An extensive numerical study is made of the nonlinear deformations of the shells. We find for example, that all solution branches that bifurcate from the eigenvalues of the linearized buckling theory are connected to each other by means of intermediate branches. Some implications of the numerical results concerning the buckling of spherical shells are discussed.			

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Dr. A.J. Ignatowski  
Redstone Arsenal Research Div.  
Rohm & Haas Company  
Huntsville, Alabama 35807

Dr. M.L. Merritt  
Division 5412  
Sandia Corporation  
Sandia Base  
Albuquerque, New Mexico 87115

Director  
Ship Research Institute  
Ministry of Transportation  
700, SHINKAWA  
Mitaka  
Tokyo, Japan

Dr. H. N. Abramson  
Southwest Research Institute  
8500 Culebra Road  
San Antonio, Texas 78206

Industry and Research Inst. (cont'd.)

Dr. R. C. DeHart  
Southwest Research Institute  
8500 Culebra Road  
San Antonio, Texas 78206

Stanford University  
Mathematics Department  
Stanford, California 94305

Dr. Thor Smith  
Stanford Research Institute  
Menlo Park, California 94025

Massachusetts Institute of  
Technology  
Mathematics Department  
Room 2-247  
Cambridge, Massachusetts 02139

Dr. M. L. Baron  
Paul Weidlinger, Consulting Engr.  
777 Third Ave. - 22nd Floor  
New York, New York 10017

Dr. T. C. Fan  
The Rand Corporation  
1700 Main Street  
Santa Monica, California 90406

Dr. G. R. Makepeace  
Director, Research and Engineering  
Lockheed Propulsion Company  
P.O. Box 111  
Redlands, California 92374

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Director, Naval Research Laboratory (5)  
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(Code 432)  
Washington, D.C. 20360







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Bauer

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AUTHOR

Axisymmetric buckling of

TITLE

hollow spheres...

DATE DUE

BORROWER'S NAME

N.Y.U. Courant Institute of  
Mathematical Sciences

251 Mercer St.

New York, N. Y. 10012

